

ABBREVIATIONS
AB ANCHOR BOLT
ABV ABOVE
ADJ ADJACENT
AFP ABOVE FINISH FLOOR
AGR AGGREGATE
ALT ALTERNATE
ALUM ALUMINUM
APPROX APPROXIMATELY
ARCHL ARCHITECTURAL
BD BOARD
BLDG BUILDING
BLKG BLOCKING
BM BEAM
BN BOUNDARY NAILING
BO BOTTOM OF
BOV BOTTOM
BRT BEARING
BTWN BETWEEN
CAM CAMBER
CANTL CANTILEVERED
CTR CENTERED
CL CENTERLINE
CLR CLEAR
CMU CONC MASONRY UNIT
COL COLUMN
CONC CONCRETE
CONN CONNECTION
CONST CONSTRUCTION
CONT CONTINUOUS
C PENNY
DI DIA DIAMETER
DBL DOUBLE
DEPT DEPARTMENT
DPL DOUGLAS FIR LARCH
DIAG DIAGONAL
DTL DETAIL
DIA DIAMETER
DM DIMENSION
DN DOWN
DWG DRAWING
DWL DOWEL
EA EACH
EJ EXPANSION JOINT
ELEV ELEVATION
ELEC ELECTRIC
EN EDGE NAILING
ENCL ENCLOSED
EOR ENGINEER OF RECORD
EQ EQUAL
EQUIP EQUIPMENT
EXIST EXISTING
EXP EXPANSION
EXT EXTERIOR
EXTD EXTENDED
FDD FLOOR DRAIN
FOUND FOUNDATION
FF FINISH FLOOR
FG FINISH GRADE
FIN FINISH
FLR FLOOR
FRMG FRAMING
FT FOOT OR FEET
FTG FOOTING
FLRR FURRING
GA GAUGE
GLB GLU-LAM BEAM
GRND GROUND
GR GIRDER TRUSS
GYP GYPSUM WALLBOARD
HDR HEADER
HGR HANGER
HGT HT HEIGHT
HORZ HORIZONTAL
HTR HIP TRUSS
INSUL INSULATION
INT INTERIOR
JT JOINT
LAM LAMINATE
LAT LATERAL
LBS POUNDS
LSL LAM STRAND LUMBER
LT LIGHT
LVL LAM VENEER LUMBER
LV LONG WAY
MAS MASONRY
MAX MAXIMUM
MECH MECHANICAL
MFR MANUFACTURER
MFRD MANUFACTURED
MIN MINIMUM
MISC MISCELLANEOUS
NO NUMBER
N COMMON NAIL
NTS NOT TO SCALE
OV OVER
OC ON CENTER
OH OPPOSITE HAND
OPNG OPENING
PFA POST FROM ABOVE
PL PLATE
PLYWD PLYWOOD
PNL PANEL
PREFAB PREFABRICATED
PSL PARALLEL STRAND LUMBER
PSI LBS PER SQ INCH
PT POST-TENSIONED
P POUND
RAD RADIUS
REF REFERENCE
REINF REINFORCING
REQD REQUIRED
S SINKER NAIL
SCHED SCHEDULE
SF SQUARE FEET
SECT SECTION
SHTHG SHEATHING
SM SIMILAR
SPEC SPECIFICATIONS
SQ SQUARE
STD STANDARD
STGD STAGGERED
STL STEEL
STRUCT STRUCTURAL
SYM SYMMETRICAL
SWL SHEAR WALL
SPN SOLE PLATE NAILING
SW SHORT WAY
T&G TONGUE AND GROOVE
T&B TOP AND BOTTOM
THK THICK
TO TOP OF
TRIMM TRIMMER
TR TRUSS
TS TUBE STEEL
TS LSL TIMBERSTRAND BEAM
TYP TYPICAL
UNO UNLESS NOTED OTHERWISE
VERT VERTICAL
W/ W/O WITH WITHOUT
WS WOOD SCREW
WT WEIGHT

SIMPSON

STRUCTURAL GENERAL NOTES

GENERAL

- ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING CODE: THE INTERNATIONAL BUILDING AND RESIDENTIAL CODE, 2018 EDITION, OTHER REGULATING AGENCIES WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK, AND THOSE CODES AND STANDARDS LISTED IN THESE NOTES AND SPECIFICATIONS.
- THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE ABOVE ITEMS.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.
- DIMENSIONS SHALL TAKE PRECEDENCE OVER THE SCALE SHOWN ON DRAWINGS.
- NOTES AND DETAILS ON PLANS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
- SEE ARCHITECTURAL PLANS FOR THE FOLLOWING UNO:
 - SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS.
 - SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-BEARING PARTITIONS.
 - SIZE AND LOCATION OF ALL CONCRETE CURBS, FLOOR DRAINS, SLOPES, DEPRESSIONED AREAS, CHANGES IN LEVEL, CHAMFER, GROOVES, INSERTS, ETC.
 - SIZE AND LOCATION OF FLOOR AND ROOF OPENINGS.
 - FLOOR AND ROOF FINISHES.
 - STAIR FRAMING AND DETAILS.
 - DIMENSIONS NOT SHOWN ON STRUCTURAL PLANS.
- SEE MECHANICAL, PLUMBING, AND ELECTRICAL PLANS FOR THE FOLLOWING:
 - PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., (EXCEPT AS SHOWN OR NOTED).
 - ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.
 - CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL, OR PLUMBING FIXTURES.
 - SIZE AND LOCATION OF MACHINE EQUIPMENT BASES, OR ANCHOR BOLTS FOR MOUNTS.
 - SIZE AND LOCATION OF ALL MECHANICAL UNITS.
- OPENINGS, POCKETS, ETC. LARGER THAN 6 INCHES SHALL NOT BE PLACED IN SLABS, DECKS, BEAMS, JOISTS, COLUMNS, WALLS, ETC., UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.
- ASTM SPECIFICATIONS NOTED SHALL BE THE LATEST REVISION
- THE CONTRACTOR SHALL INVESTIGATE THE SITE DURING CLEARING AND EARTHWORK OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC. IF ANY SUCH STRUCTURES ARE FOUND, THE ENGINEER OF RECORD SHALL BE NOTIFIED IMMEDIATELY.
- CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FLOORS OR ROOF. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE STRUCTURE HAS NOT ATTAINED DESIGN STRENGTH.
- WHERE THE LONGEST HORIZONTAL CEILING DIMENSION IS EQUAL TO OR GREATER THAN 20'-0", IT IS RECOMMENDED THAT RESILIENT CHANNEL BE USED TO HELP LIMIT DRYWALL CRACKING.

DESIGN CRITERIA

	LIVE LOAD	DEAD LOAD
FLOOR LOAD	40 PSF	15 PSF
ROOF LOAD	20 PSF	22 PSF
ROOF SNOW LOAD	≤ 10 PSF	22 PSF
STAIR & EXIT LOAD	---	---
STORAGE LOAD	---	---
★ SEE FRAMING NOTES FOR SPECIAL LOADING CONDITIONS.		
BASIC WIND SPEED	100 MPH	
WIND RISK CATEGORY	II	
WIND EXPOSURE CATEGORY	C	
INTERNAL PRESSURE COEFFICIENT	± 0.18	
SEISMIC RISK CATEGORY & IMPORTANCE	II	I _e = 1.0
MAPPED SPECTRAL RESPONSE ACCELERATIONS	S _{DS} = 0.486 & S ₁ = 0.163	
SPECTRAL RESPONSE COEFFICIENTS	S _{DS} = 0.421 & S _{DS1} = 0.163	
SITE CLASS	C	
LATERAL FORCE RESISTING SYSTEM	LIGHT FRAME WALLS WITH SHEAR PANELS	R = 6.5
SEISMIC RESPONSE COEFFICIENT, C _s	0.065	
DESIGN BASE SHEAR	C _s x DEAD WEIGHT(W)	
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE	
SEISMIC DESIGN CATEGORY	C	

FOUNDATION

- THIS IS TO CERTIFY THE FOUNDATION DEPICTED HEREIN HAS BEEN DESIGNED IN ACCORDANCE WITH RECOGNIZED ENGINEERING PRACTICE FOR CONDITIONS AS CLASSIFIED BY THE PROJECT GEOTECHNICAL REPORT.

CONSULTANT: DUPONT ENGINEERING, INC
PROJECT NO: 19-0457
DATED: JUNE 30, 2019
UPDATED:
- FOOTINGS ARE DESIGNED BASED ON AN ALLOWABLE BEARING PRESSURE OF 2000 PSF PER SOIL REPORT.
- SOILS PREPARATION AND FOUNDATION CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE GEOTECHNICAL REPORT.
- SEE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR EXACT LOCATION OF BULKHEADS AND OPENINGS, ETC.
- THE CONTRACTOR SHALL PROVIDE FOR PROPER DE-WATERING OF EXCAVATIONS FROM SURFACE WATER, GROUND WATER, SEEPAGE, ETC. (FOOTINGS SHALL NOT BE PLACED UNDER WATER).
- FOOTINGS SHALL BE PLACED ACCORDING TO DEPTHS SHOWN ON THE STRUCTURAL PLANS. ALL ABANDONED FOOTINGS, UTILITIES, ETC. THAT INTERFERE WITH NEW CONSTRUCTION SHALL BE REMOVED.
- CONCRETE PLACEMENT SHALL BE IN ONE CONTINUOUS OPERATION UNLESS OTHERWISE SPECIFIED AND SLAB SURFACE SHALL BE CURED WITH HUNTS COMPOUND OR EQUAL.
- FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN BUILDING AREA SHALL BE PER THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. FLOODING WILL NOT BE PERMITTED.
- STOOPS, PORCHES, OR OTHER ATTACHMENTS SHALL BE CAST INDEPENDENT OF THE CONCRETE FOUNDATION SLAB, UNO.

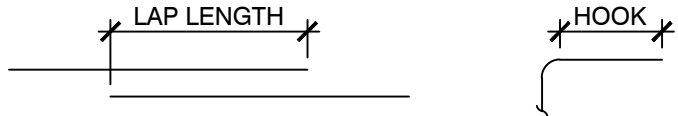
FOUNDATION HARDWARE

- THICKEN SLAB AS REQUIRED FOR CONCRETE COVERAGE AT ANCHOR BOLTS PER ACI 318 WHERE OCCURS. THE CONCRETE CONTRACTOR SHALL VERIFY LOCATION OF ALL BOLTS, TIE-DOWNS, POST-ANCHORS, ETC. WITH THE ARCHITECTURAL PLANS PRIOR TO COMMENCING WORK AND BE RESPONSIBLE FOR SAME.
- UNO BOLT SILL PLATES TO THE FOUNDATIONS WITH MIN 1/2" NOMINAL DIA ANCHOR BOLT WITH PLATE WASHERS. BOLTS SHALL BE SPACED NOT MORE THAN 72" O.C. THERE SHALL BE A MIN OF (2) BOLTS PER PIECE. ONE BOLT SHALL BE LOCATED NOT MORE THAN 12" AND NOT LESS THAN 4" FROM EACH END, OR FROM EACH SIDE OF A NOTCH GREATER THAN 1/2 THE WIDTH OF THE PLATE. EMBED BOLTS AT LEAST 7" INTO REINFORCED MASONRY OR CONCRETE. SILL PLATE ANCHOR BOLTS MAY BE "WET SET".
- PROVIDE 3" X 3" X 0.229" PLATE WASHERS ON ALL ANCHOR BOLTS AT SHEAR WALLS. PLATE WASHERS SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE SILL PLATE ON THE SIDE(S) WITH SHEATHING. PROVIDE STANDARD CUT WASHERS UNDER BOLT HEADS AND NUTS WHEN SLOTTED PLATE WASHERS ARE USED. PROVIDE CUT OR SLOTTED WASHERS AGAINST WOOD AT ALL REMAINING WALLS.
- ALL 1/2" DIA. ANCHOR BOLTS MAY BE REPLACED WITH:
 - 1/2" DIA. THREADED ROD EPOXY DOWELED TO FOUNDATION WITH SIMPSON SET-UP EPOXY (ICC REPORT ESR-2508) IN 5/8" DIA. HOLE. PROVIDE A MIN. OF 4" EMBEDMENT AND 1 3/4" EDGE DISTANCE FROM ANY SLAB EDGE.
 - 1/2" DIA SIMPSON TITEN HD HIGH STRENGTH THREADED ANCHOR (ICC ESR-2713). PROVIDE A MIN. OF 4" EMBEDMENT AND 1 3/4" EDGE DISTANCE FROM ANY SLAB EDGE.
 - SIMPSON MASSASAP MUDSILL ANCHORS (ICC ESR-2555). INSTALLATION OF THE MUDSILL ANCHOR SHALL BE PER MANUFACTURERS TYPICAL OR ALTERNATE INSTALLATION METHOD. DO NOT WET-SET THE MUDSILL ANCHOR. INSTALL WITH ONE LEG UP OR INSTALL OVER PLYWOOD OR OSB. FOLLOW ALL MANUFACTURERS SPECIFICATIONS AND RECOMMENDATIONS DURING INSTALLATION. DO NOT US WHEN ANCHOR SPACING IS ≤ 9 INCHES.
- AT INTERIOR NON-BEARING AND NON-SHEARWALLS, 1/2" DIA ANCHOR BOLTS MAY BE REPLACED WITH MINIMUM 0.145" DIA X 2 1/8" POWDER DRIVEN FASTENERS AT A MAXIMUM SPACING OF 24" O.C. (HLTI ESR-2379 OR SIMPSON ESR-2138 OR EQUIVALENT)
- PROVIDE REINFORCING BAR FOR HOLDOWNS PER DETAILS SHEET SD-1, UNO.
- HOLDOWNS SPECIFIED ON PLANS MAY BE POST INSTALLED (RETRO-FITTED) AS NOTED BELOW. DRILL COMPLETELY THRU THE FOOTING AND PROVIDE THREADED WITH BEARING PLATE, WASHER AND NUT. BACKFILL ALL AROUND THE ROD, WASHER AND NUT TO PROVIDE A MINIMUM OF 3" COVER USING LEAD CONCRETE MIX. THREADED ROD AND BEARING PLATE SHALL BE PER THE TABLE PROVIDED. SEE RETROFIT DETAIL SHEET SD-1.1. THE USE OF POST INSTALLED ANCHORS UNLESS NOTED OTHERWISE, E.G. EXPANSION ANCHORS OR EPOXY ANCHORS, IS NOT ALLOWED WITHOUT PRIOR REVIEW AND AUTHORIZATION BY THE ENGINEER OF RECORD. CONTACT THE ENGINEER OF RECORD FOR REQUEST OF ALTERNATE ANCHORAGE SOLUTIONS.

REINFORCING STEEL

- REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-615 GRADE 60.
- WELDED REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-706 GRADE 60.
- ALL REINFORCING BAR BENDS SHALL BE MADE COLD.
- WELDED WIRE REINFORCEMENT (WWR) SHALL BE PER ASTM A185 AND SHALL BE SUPPLIED IN SHEETS. WWR FRM ROLLS SHALL NOT BE USED.
- MINIMUM LAP OF WELDED WIRE FABRIC SHALL BE 6 INCHES OR ONE AND ONE HALF SQUARES, WHICHEVER IS GREATER.
- ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN-PLACE INSPECTION IS MADE.
- REBAR SPICES ARE TO BE CLASS "B" (UNO). MAINTAIN 2 BAR DIA CLEAR SPACE BETWEEN ADJACENT SPICES.
- REINFORCING SPICES SHALL BE MADE ONLY WHERE INDICATED ON THE DRAWINGS.
- DOWELS BETWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE AND SPACING OR NUMBER AS THE VERTICAL REINFORCING, RESPECTIVELY, UNO.

REBAR SIZE	3	4	5	6	7	8	9	10	11
MINIMUM LAP LENGTH (INCHES)	22	29	36	43	68	78	88	98	107
STD HOOK LENGTH (INCHES)	4 1/2	6	7 1/2	9	10 1/2	12	13 1/2	15 1/2	16 1/2



CONCRETE

- ALL PHASES OF WORK PERTAINING TO THE CONCRETE CONSTRUCTION SHALL CONFORM TO THE 'BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE' (ACI 318 LATEST ADOPTED EDITION), WITH MODIFICATIONS AS NOTED IN THE DRAWINGS AND SPECIFICATIONS.
- REINFORCED CONCRETE DESIGN IS BY THE 'ULTIMATE STRENGTH DESIGN METHOD'. SCHEDULE OF STRUCTURAL CONCRETE 28-DAY STRENGTHS AND TYPES:

LOCATION IN STRUCTURE	STRENGTH	PSI TYPE
SLABS ON GRADE	2500 PSI	HARD ROCK
FOOTINGS	2500 PSI	HARD ROCK

DESIGN BASED ON 2500 PSI, 28-DAY STRENGTH. THEREFORE SPECIAL INSPECTION IS NOT REQUIRED.
- CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL WITH THE FOLLOWING REQUIREMENTS:
 - COMPRESSIVE STRENGTH AT AGE 28 DAYS AS SPECIFIED ABOVE.
 - LARGE AGGREGATE-HARDROCK: 3/4" MAXIMUM SIZE CONFORMING TO ASTM C-33.
 - CEMENT: ASTM C-150, TYPE V PORTLAND CEMENT.
 - MAXIMUM SLUMP: 5 INCHES.
 - NO ADMIXTURES, EXCEPT FOR ENTRAINED AIR, AND AS APPROVED BY THE ENGINEER.
 - WATER/CEMENT RATIO SHALL BE AS FOLLOWS:
 - fc = 4500 PSI.....w/c = 0.45
 - fc = 4000 PSI.....w/c = 0.5
 - fc < 4000 PSI.....w/c = PER MIX DESIGN
 - FLY ASH: ASTM C818
 - SLAG: ASTM C989
 - SILICA FUME: ASTM C1240
 - LIGHTWEIGHT AGGREGATE: ASTM C330
 - AIR ENTRAINING ADMIXTURE: ASTM C660
 - WATER REDUCERS: ASTM C494, TYPE A OR F
- CONCRETE MIXING OPERATIONS, ETC. SHALL CONFORM TO ASTM C-94.
- PLACEMENT OF CONCRETE SHALL CONFORM TO ACI STANDARD 614 AND PROJECT SPECIFICATIONS.
- CLEAR COVERAGE OF CONCRETE OVER OUTER REINFORCING BARS SHALL BE AS FOLLOWS:
 - CONCRETE POURED DIRECTLY AGAINST EARTH: 3 INCHES CLEAR
 - STRUCTURAL SLABS: 3/4 INCHES CLEAR, TOP AND BOTTOM (2" TOP, 3/4" BOTTOM IN CORROSIVE ENVIRONMENTS)
 - FORMED CONCRETE WITH EARTH BACKFILL: 2 INCHES CLEAR
- ALL REINFORCING BARS, HOLD DOWN BOLTS AND STRAPS, AND OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.
- PROVIDE SLEEVES FOR PLUMBING AND ELECTRICAL OPENINGS IN CONCRETE BEFORE PLACING. DO NOT CUT ANY REINFORCING WHICH MAY CONFLICT. CORING IN CONCRETE IS NOT PERMITTED EXCEPT AS SHOWN. NOTIFY THE STRUCTURAL ENGINEER IN ADVANCE OF CONDITIONS NOT SHOWN ON THE DRAWINGS.
- CONDUIT OR PIPE SIZE (O.D.) SHALL NOT EXCEED 30% OF SLAB THICKNESS AND SHALL BE PLACED BETWEEN THE TOP AND BOTTOM REINFORCING, UNLESS SPECIFICALLY DETAILED OTHERWISE. CONCENTRATIONS OF CONDUITS OR PIPES SHALL BE AVOIDED EXCEPT WHERE DETAILED OPENINGS ARE PROVIDED.
- MODULUS OF ELASTICITY OF CONCRETE, WHEN TESTED IN ACCORDANCE WITH ASTM C-460, SHALL BE AT LEAST THE VALUE GIVEN BY THE EQUATIONS IN SECTION 8.5.1 OF ACI 318 FOR THE SPECIFIED 28-DAY STRENGTH.
- SEE FOUNDATION DETAILS FOR REINFORCEMENT REQUIRED AT CORNERS AND INTERSECTIONS OF CONCRETE WALLS, CONVENTIONAL FOOTINGS AND GRADE BEAMS.

WOOD

- FRAMING LUMBER, UNO:
 - 2X AND 4X TO BE DOUGLAS FIR LARCH (DFL) NO. 2 GRADE, SEE FRAMING PLANS AND NOTES FOR WALL STUD REQUIREMENTS.
 - 6X TO BE DOUGLAS FIR LARCH NO. 1 GRADE.
 - ALL LUMBER SHALL HAVE A MOISTURE CONTENT OF LESS THAN 19%.
- BOLT HOLES SHALL BE 1/16" (MAXIMUM) LARGER THAN THE BOLT SIZE. RE-TIGHTEN ALL NUTS PRIOR TO CLOSING IN.
- ALL SILLS OR PLATES RESTING ON CONCRETE OR MASONRY SHALL BE PRESSURE TREATED DOUGLAS FIR USING BORON BASED PRESERVATIVES OR LSL TREATED WITH ZINC BORATE.
- DO NOT NOTCH JOISTS, RAFTERS, OR BEAMS, EXCEPT WHERE SHOWN IN DETAILS. OBTAIN ENGINEERS APPROVAL FOR ANY HOLES OR NOTCHES NOT DETAILED. (I.E. ALL POSTS TO BE CONTINUOUS UNTIL FULLY SUPPORTED BY BEAM OR FOUNDATION BELOW).
- WHERE A POST OCCURS AT AN UPPER LEVEL, ADD THE SAME POST DIRECTLY BELOW IT ON THE LOWER LEVELS AND IN BETWEEN FLOOR SHEATHING AND LOWER LEVEL WALL TOP PLATES. ALL POSTS TO BE CONTINUOUS UNTIL FULLY SUPPORTED BY BEAM OR FOUNDATION BELOW.
- FACE NAIL EACH PLY OF MULTIPLE 2X POSTS WITH 16D SINKERS AT 6" O.C.
- PROVIDE MULTIPLE 2X POST AT ALL GIRDER TRUSS AND BEAM BEARING LOCATIONS, WIDTH TO MATCH BEAM OR TRUSS, MIN. (2) 2X UNO.
- CONNECTION HARDWARE SHALL BE SIMPSON OR EQUAL AND MUST BE I.C.C. APPROVED.
- FOR CONNECTIONS NOT DETAILED, PROVIDE FASTENERS PER TABLE 2304.9.1 OF THE LATEST ADOPTED EDITION OF THE INTERNATIONAL BUILDING CODE.
- DO NOT NOTCH TOP PLATES OR STUDS EXCEPT AS SHOWN IN DETAILS. OBTAIN ENGINEERS APPROVAL FOR ANY HOLES OR NOTCHES NOT DETAILED.
- NON-BEARING WALLS SHALL HAVE STUDS SPACED AT 24" O.C. (MAX). REFER TO ARCHITECTURAL DRAWINGS FOR SIZE. TOP PLATES SHALL BE SUCH THAT A 1/2" GAP BETWEEN THE TOP OF THE PLATES AND THE BOTTOM OF THE TRUSSES AND/OR BLOCKING PANELS EXISTS AFTER THE ROOF IS LOADED.
- TYPICAL FASTENER SIZE NOTED IN DRAWINGS:
8d COMMON NAILS = 0.131" DIA X 2 1/2", 10d COMMON NAILS = 0.148" DIA
16d COMMON NAILS = 0.162" DIA 16d SIMPSON NAILS = 0.148" DIA X3 1/4".
- 16d SINKERS NOTED IN THESE DRAWINGS MAY BE REPLACED WITH 0.131 X 3 1/4" GUN NAILS PER THE FOLLOWING TABLES:

STANDARD TRUSSES ⁽²⁾		
REACTIONS TO THE BEARING WALL OR BEAM BELOW:		
TRUSS TYPE & TOTAL LENGTH	HARDWARE REQUIRED	NAILING REQUIRED
JACK TRUSSES ≤ 10'-0"	NONE REQUIRED	UPLIFT IS RESISTED BY EXISTING NAILED CONNECTION OF TRUSS TO PLATE ⁽¹⁾
HIP/COMMON TRUSSES ≤ 30'-0"	NONE REQUIRED	UPLIFT IS RESISTED BY EXISTING NAILED CONNECTION OF TRUSS TO PLATE ⁽¹⁾
HIP/COMMON TRUSS 30'-0" ≤ L ≤ 50'-0"	H1 OR H2.5	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
ALL HIP/COMMON DRAG TRUSSES L ≤ 50'-0"	H1 OR H2.5	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
HIP/COM. GIRDER 8FT SETBACK L ≤ 30'-0"	H10 OR (2) H2.5	(12) 10d x 1 1/2" INTO TRUSS (12) 10d x 1 1/2" INTO PLATE
HIP/COM. GIRDER 8FT SETBACK L ≤ 40'-0"	HTS20	(12) 10d x 1 1/2" INTO TRUSS (12) 10d x 1 1/2" INTO PLATE & STUD BLW
HIP/COM. GIRDER 8FT SETBACK L ≤ 50'-0"	(2) HTS20	(12) 10d x 1 1/2" INTO TRUSS (12) 10d x 1 1/2" INTO PLATE & STUD BLW
ALL OTHER NON-DRAG GIRDER TR L ≤ 40'-0"	H1 OR H2.5	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
ALL OTHER GIRDER TRUSSES L ≤ 50'-0"	H10 OR (2) H2.5	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE

AT TRUSSES WHERE THE HARDWARE ABOVE CANNOT BE INSTALLED, PROVIDE AN HTS20 STRAP

GABLE END WALL TRUSSES	
HARDWARE	NAILING REQUIRED
LTH4 AT 32" OC UNO	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE

- NOTES:
- SEE DETAIL 1/SD-3 FOR STANDARD NAILING REQUIREMENTS
 - SEE FRAMING PLANS FOR NON-STANDARD UPLIFT HARDWARE

MANUFACTURED BEAMS

- GLUE LAMINATED BEAMS (GLB)
 - GLB SHALL BE 24F-V4, (CONTINUOUS AND CANTILEVERED GLB SHALL BE 24F-V8) AND HAVE THE FOLLOWING MINIMUM PROPERTIES: FB=2400 PSI, FV=240 PSI
FC (PERPENDICULAR) = 650 PSI
E=1,800,000 PSI
 - ALL BEAMS SHALL BE FABRICATED USING EXTERIOR GLUE, FABRICATION AND HANDLING PER LATEST ATC AND WCCA STANDARDS. BEAMS TO BEAR GRADE STAMP AND ATC STAMP AND CERTIFICATE.
 - MOISTURE CONTENT SHALL BE LIMITED TO A MAXIMUM OF 12%.
 - ALL GLB SHALL HAVE STANDARD CAMBER, UNO, ON PLANS.
- LAMINATED STRAND LUMBER (1.3E OR 1.5E LSL)
 - LSL BEAMS SHALL HAVE I.C.C. APPROVAL AND HAVE THE FOLLOWING MINIMUM PROPERTIES:
 - LSL (1.3E) - E = 1,300,000 PSI, FB = 1700 PSI, FV = 400 PSI
FC (PERPENDICULAR) = 680 PSI, FC (PARALLEL) = 1400 PSI
 - LSL (1.5E) - E = 1,500,000 PSI, FB = 2250 PSI, FV = 400 PSI
FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 1950 PSI
 - ALL MULTI-PLY LSL MEMBERS SPECIFIED ON PLANS MAY BE REPLACED WITH SOLID MEMBERS OF EQUAL OR GREATER PROPERTIES WITH EQUAL OR GREATER WIDTH AND DEPTH WITHOUT FURTHER REVIEW.
- LAMINATED VENEER LUMBER (LVL)
 - LVL BEAMS SHALL HAVE I.C.C. APPROVAL AND HAVE THE FOLLOWING MINIMUM PROPERTIES:
 - LVL (1.7E) - E = 1,700,000 PSI, FB = 2650 PSI, FV = 285 PSI
FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 3000 PSI
 - LVL (1.9E) - E = 1,900,000 PSI, FB = 2800 PSI, FV = 285 PSI
FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 2910 PSI
 - LVL (2.0E) - E = 2,000,000 PSI, FB = 2800 PSI, FV = 285 PSI
FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 3000 PSI
 - MULTIPLE-PLY LVL BEAMS SHALL BE NAILED TOGETHER AS FOLLOWS:
 - PROVIDE (2) ROWS OF 16D SINKERS AT 12" O.C.
FOR BEAMS ≤ 11'10" DEEP
 - PROVIDE (3) ROWS OF 16D SINKERS AT 12" O.C.
FOR BEAMS > 11'10" DEEP.
 - ALL MULTI-PLY LVL MEMBERS SPECIFIED ON PLANS MAY BE REPLACED WITH SOLID MEMBERS OF EQUAL OR GREATER PROPERTIES WITH EQUAL OR GREATER WIDTH AND DEPTH WITHOUT FURTHER REVIEW.
- PARALLEL STRAND LUMBER (PSL)
 - PSL BEAMS SHALL HAVE I.C.C. APPROVAL AND HAVE THE FOLLOWING MINIMUM PROPERTIES:
E = 2,000,000 PSI, FB = 2900 PSI, FV = 290 PSI
FC (PERPENDICULAR) = 1550 PSI, FC (PARALLEL) = 2900 PSI

PREFABRICATED WOOD TRUSSES

- PREFABRICATED WOOD ROOF TRUSSES SHALL BE AS DESIGNED BY THE TRUSS MANUFACTURER (INCLUDING BRIDGING SIZE AND SPACING) UNO. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, ERECTION DRAWINGS, AND DESIGN CALCULATIONS, SEALED BY AN ENGINEER, REGISTERED IN THE STATE OF THE GOVERNING JURISDICTION, FOR REVIEW PRIOR TO MANUFACTURE. CALCULATIONS AND SHOP DRAWINGS SHALL SHOW ANY SPECIAL DETAILS REQUIRED AT BEARING POINTS. ALL CONNECTORS SHALL HAVE CURRENT I.C.C. APPROVAL.
- TRUSS MANUFACTURER TO DESIGN TRUSSES FOR LATERAL LOAD (LAT. = XXXX) IN POUNDS, AS SHOWN ON PLANS.
- UNLESS OTHERWISE NOTED, TRUSSES TO BE DESIGNED FOR LOADS INDICATED IN THE DESIGN CRITERIA AND AS FOLLOWS:
 - TOP CHORD DEAD LOAD = XXX + ADDITIONAL 5 PSF AT OVERFRAMING.
 - BOTTOM CHORD DEAD LOAD = XXX.
 - WIND LOAD: WHEN CALCULATING NET UPLIFT REACTIONS, USE MAXIMUM RESISTING DEAD LOAD = XXX PSF ON TOP CHORD AND XXX PSF ON BOTTOM CHORD
- TRUSS MANUFACTURER TO DESIGN TRUSSES TO SUPPORT MECHANICAL EQUIPMENT AS REQUIRED. ADDITIONAL TRUSSES MAY BE SUPPLIED AS REQUIRED.
- ALL TRUSS TO TRUSS CONNECTORS PER TRUSS MANUFACTURER.
- WHERE POST OCCURS ABOVE A MANUFACTURED TRUSS, A VERTICAL WEB SHALL BE PROVIDED UNDER THE POST. THE WEB SECTION AREA SHALL BE EQUAL TO OR LARGER THAN POST SECTION AREA. MULTIPLE 2XHS MAY BE USED FOR THE VERTICAL WEB.
- THE TOP CHORD OF ALL TRUSSES SHALL HAVE A SPECIES WITH SPECIFIC GRAVITY EQUAL TO OR GREATER THAN 0.42.
- TRUSS MANUFACTURER SHALL LIMIT TOTAL LOAD DEFLECTIONS TO LESS THAN LOAD AND LIVE LOAD DEFLECTIONS TO LESS THAN L/360. DEFLECTION SHALL BE LIMITED SO AS NOT TO CREATE A BEARING CONDITION AT NON-BEARING WALLS. REFER ALSO TO NOTE 11 OF THE WOOD SECTION.
- CONNECTION OF MANUFACTURED TRUSSES FOR UPLIFT SHALL BE PER THE TABLES ON SHEET S1.1.
- TRUSSES SHALL BE DESIGNED TO SUPPORT A 250 POUND CONCENTRATED LOAD AT ANY LOCATION ALONG THE BOTTOM CHORD.

STANDARD TRUSSES ⁽²⁾		
REACTIONS TO THE BEARING WALL OR BEAM BELOW:		
TRUSS TYPE & TOTAL LENGTH	HARDWARE REQUIRED	NAILING REQUIRED
JACK TRUSSES ≤ 10'-0"	NONE REQUIRED	UPLIFT IS RESISTED BY EXISTING NAILED CONNECTION OF TRUSS TO PLATE ⁽¹⁾
HIP/COMMON TRUSSES ≤ 30'-0"	NONE REQUIRED	UPLIFT IS RESISTED BY EXISTING NAILED CONNECTION OF TRUSS TO PLATE ⁽¹⁾
HIP/COMMON TRUSS 30'-0" ≤ L ≤ 50'-0"	H1 OR H2.5	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
ALL HIP/COMMON DRAG TRUSSES L ≤ 50'-0"	H1 OR H2.5	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
HIP/COM. GIRDER 8FT SETBACK L ≤ 30'-0"	H10 OR (2) H2.5	(12) 10d x 1 1/2" INTO TRUSS (12) 10d x 1 1/2" INTO PLATE
HIP/COM. GIRDER 8FT SETBACK L ≤ 40'-0"	HTS20	(12) 10d x 1 1/2" INTO TRUSS (12) 10d x 1 1/2" INTO PLATE & STUD BLW
HIP/COM. GIRDER 8FT SETBACK L ≤ 50'-0"	(2) HTS20	(12) 10d x 1 1/2" INTO TRUSS (12) 10d x 1 1/2" INTO PLATE & STUD BLW
ALL OTHER NON-DRAG GIRDER TR L ≤ 40'-0"	H1 OR H2.5	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
ALL OTHER GIRDER TRUSSES L ≤ 50'-0"	H10 OR (2) H2.5	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE

AT TRUSSES WHERE THE HARDWARE ABOVE CANNOT BE INSTALLED, PROVIDE AN HTS20 STRAP

GABLE END WALL TRUSSES	
HARDWARE	NAILING REQUIRED
LTH4 AT 32" OC UNO	(8) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE

- NOTES:
- SEE DETAIL 1/SD-3 FOR STANDARD NAILING REQUIREMENTS
 - SEE FRAMING PLANS FOR NON-STANDARD UPLIFT HARDWARE

SHEET INDEX

SHT #	SHEET NAME
S1	GENERAL NOTES
S1.1	SCHEDULES AND NOTES
S2	FOUNDATION PLAN
S3	ROOF FRAMING PLANS
S4	SHEAR WALL PLANS
SD-1	STRUCTURAL DETAILS
SD-1.1	STRUCTURAL DETAILS
SD-2	STRUCTURAL DETAILS
SD-2.1	STRUCTURAL

SHEARWALL SCHEDULE ^(2, 5, 8, 11, 12, 13)					
SHEARWALL ^(7, 17)			ONE SIDED SHEARWALL TWO SIDED SHEARWALL ⁽⁶⁾		SEISMIC WIND
MARK	MATERIAL ^(4, 14)	NAILING	UPPER FLOOR SILL PL CONN ^(1, 18)	ANCHOR BOLTS ⁽³⁾ (UNO ON FOUNDATION PLAN)	ALLOWABLE SHEAR
P1	3/8" APA SHEATHING ⁽¹⁵⁾	8dN AT 6" OC EDGES, 12" FIELD ⁽¹⁶⁾	16dS AT 4" OC (STG'D) UNO	1/2" DIA X 10" AT 32" OC	260#/FT 365#/FT
P2	3/8" APA SHEATHING ⁽¹⁵⁾	8dN AT 4" OC EDGES, 12" FIELD ⁽¹⁶⁾	16dS AT 3" OC, (STG'D) UNO	1/2" DIA X 10" AT 24" OC	350#/FT 532#/FT
P3	3/8" APA SHEATHING ^(9, 15)	8dN AT 3" OC EDGES, 12" FIELD ⁽¹⁶⁾	1/4x6 SCREWS AT 4" OC (STG'D), UNO	1/2" DIA X 10" AT 16" OC	490#/FT 685#/FT
P4	3/8" APA SHEATHING ^(9, 15)	8dN AT 2" OC EDGES, 12" FIELD ⁽¹⁶⁾	1/4x6 SCREWS AT 4" OC (STG'D), UNO	1/2" DIA X 10" AT 12" OC	600#/FT 895#/FT
P5	15/32" APA SHEATHING ⁽¹⁰⁾	10dN AT 2" OC EDGES, 12" FIELD ⁽¹⁶⁾	1/4x6 SCREWS AT 3" OC (STG'D), UNO	1/2" DIA X 10" AT 9" OC	770#/FT 1078#/FT
SCHEDULE NOTES:					
(1) SEE DETAIL 7/SD-2 FOR ADD'TL INFO AT UPPER FLOOR SILL PLATE CONNECTION FOR MULTI-STORY PLANS ONLY. DOES NOT APPLY TO SINGLE STORY PLANS. SEE PLAN FOR SILL PLATE AND SHEAR CONNECTIONS AT EXTERIOR WALLS.					
(2) OPENINGS IN SHEARWALL SHEATHING SHALL NOT EXCEED 8 INCHES IN ANY DIRECTION FOR A SINGLE OPENING OR THE SUM OF ANY TWO OR MORE OPENINGS ON COMMON OR OVERLAPPING VERTICAL OR HORIZONTAL LINES. OPENINGS NOT GREATER THAN 8 INCHES DO NOT REQUIRE BLOCKING AROUND THE PENETRATION. CONTACT THE ENGINEER OF RECORD FOR REQUIREMENTS AT OPENINGS NOT OTHERWISE DETAILED.					
(3) MINIMUM (2) 1/2" DIA ANCHORS PER SHEAR WALL. SEE SECTION FOUNDATION HARDWARE, NOTE 2 ON THE GENERAL NOTES SHEET, S1. ALL ANCHOR BOLTS SHALL HAVE 3" x 3" x 0.229" PLATE WASHERS.					
(4) APA RATED (STRUCTURAL II) PLYWOOD OR OSB.					
(5) SEE DETAIL 11/SD-2 WHERE WALL FRAMING STEPS OR PERPENDICULAR WALL INTERSECTS SHEAR WALL.					
(6) FOR SHEAR PANELS ON TWO SIDES OF WALL, USE ONE-HALF THE SPACING GIVEN IN THE SCHEDULE FOR SILL PLATE CONNECTION AND ANCHOR BOLT SPACING, UNO.					
(7) DOUBLE SIDED SHEARWALLS SHALL HAVE VERTICAL PANEL JOINTS OFFSET TO FALL ON DIFFERENT STUDS OR USE SINGLE 3" NOMINAL STUDS (MIN) AT JOINTS. AT THE ENDS OF THE SHEARWALL, 4X NOMINAL MEMBERS ARE REQUIRED. NAILS ON EACH SIDE SHALL BE STAGGERED.					
(8) ALL SHEARWALLS REQUIRE DOUBLE 2X TOP PLATES, U.N.O. AT NON-BEARING SHEAR WALLS, SHORTEN STUDS 1/4 INCH TO PROVIDE DEFLECTION CLEARANCE.					
(9) P2, P3 AND P4 SHEARWALLS SHALL REQUIRE THE FOLLOWING: A. STAGGER NAILING ALONG PLYWOOD JOINTS AND SILL PLATES. B. SINGLE 3" NOMINAL MEMBERS AT ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS. 3" NOMINAL MEMBERS AT SINGLE SIDED SHEARWALL MAY BE CONSTRUCTED W/ (2) 2X MEMBERS FASTENED TOGETHER W/ (2) ROWS OF 16d SINKERS AT 4" OC.					
(10) P5 SHEARWALLS SHALL REQUIRE SINGLE 3" NOMINAL MEMBERS AT ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS (MIN), STAGGER JOINT AND SILL PLATE NAILING.					
(11) ALL SHEARWALL LENGTHS NOTED ON PLAN ARE MINIMUM REQUIRED AND MAY BE INCREASED WITHOUT REVIEW.					
(12) SHEATHING MAY BE PLACED ON EITHER FACE OF DESIGNATED WALL, UNO.					
(13) ALLOWABLE SHEAR CAPACITIES ARE IN ACCORDANCE WITH AF&PA SDPWS TABLE 4.3A WITH APPLICABLE OMEGA FACTORS INCLUDED.					
(14) APA SHEATHING AND GYPSUM SHEATHING MAY BE INSTALLED WITH THE LONG OR SHORT DIRECTION PERPENDICULAR TO THE FRAMING. WHERE APA SHEATHING IS INSTALLED WITH THE SHORT DIRECTION PERPENDICULAR TO THE FRAMING, THE FRAMING MUST BE 16" ON CENTER (MAX). WHERE GYPSUM SHEATHING IS INSTALLED WITH THE SHORT DIRECTION PERPENDICULAR TO THE FRAMING ALL PANEL EDGES MUST BE BLOCKED AND NAILED.					
(15) 3/8" SHEATHING MAY BE REPLACED WITH 7/16" OR 15/32" SHEATHING WITHOUT ADDITIONAL REVIEW.					
(16) ALL VERTICAL AND HORIZONTAL PANEL EDGES TO BE BLOCKED AND NAILED.					
(17) STUDS SHALL BE 24" OC (MAX). FOR 3/8" AND 7/16" SHEATHING, FIELD NAILING SHALL BE REDUCED TO 6" OC WHERE STUD SPACING IS GREATER THAN 16" OC.					
(18) SILL PLATE CONNECTORS SHALL BE PER SHEAR TRANSFER DETAIL SPECIFIED ON PLANS.					

HOLDOWN/STRAP SCHEDULE ^(1, 2)		
HD/STRAP	EMBED AT FND AND / OR ANCHOR BOLT	CONN TO (2) 2X STUD, UNO ^(3, 6, 10)
CS16	N/A	EXTEND STRAP 16" MIN. EA. END W/ (13) 8dN TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
(2) CS16	N/A	EXTEND STRAP 16" MIN. EA. END W/ (13) 8dN TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
CMSTC16	N/A	EXTEND STRAP 25" MIN. EA. END W/ (28) 16d SINKERS TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
CMST14	N/A	EXTEND STRAP 32" MIN. EA. END W/ (33) 16dN TO (3) 2X STUD ABOVE AND BELOW FLOOR FRAMING
CMST12	N/A	EXTEND STRAP 40" MIN. EA. END W/ (42) 16dN TO (3) 2X STUD ABOVE AND BELOW FLOOR FRAMING
LSTHD6 ^(4, 11)	8" EMBED	(20) 16d SINKERS
STHD10 ^(4, 11)	10" EMBED	(24) 16d SINKERS
STHD14 ^(4, 11)	14" EMBED	(30) 16d SINKERS
HTT6	SSTB24 W/ 21" EMBED ⁽¹²⁾	(26) 16dN X 2 1/2" NAILS
HOU2	SSTB24 W/ 21" EMBED ⁽¹²⁾	(6) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
HOU4	SSTB24 W/ 21" EMBED ⁽¹²⁾	(10) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
HOU5	SSTB24 W/ 21" EMBED ⁽¹²⁾	(14) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
HOU8	SSTB34 W/ 29" EMBED ⁽¹³⁾	(20) SDS 1/4 X 2 1/2 SCREWS W/ MIN (3) 2X POSTS
HOU11	1" DIA AB W/ 9" MIN EMBED W/ MIN 28" SQ X 14" DEEP FTG ⁽⁹⁾	(30) SDS 1/4 X 2 1/2 SCREWS W/ MIN 4X8 POST ⁽⁹⁾
HOU14	1" DIA AB W/ 10" MIN EMBED W/ MIN 30" SQ X 15" DEEP FTG ⁽⁷⁾	(36) SDS 1/4 X 2 1/2 SCREWS W/ MIN 4X8 POST ⁽⁹⁾
SCHEDULE NOTES:		
(1) HD/STRAP SHALL BE SIMPSON OR EQUAL W/ ICC APPROVAL. ALL SUBSTITUTES SHALL BE REVIEWED BY THE ENGINEER OF RECORD BEFORE INSTALLATION.		
(2) FIXED-LENGTH STRAPS SHALL BE INSTALLED WITH AN EQUAL LENGTH OVERLAPPING CONNECTED MEMBERS AND AN EQUAL NUMBER OF FASTENERS IN EACH MEMBER.		
(3) STITCH NAIL EACH STUD AT MULTIPLE 2x STUDS TOGETHER WITH 16d SINKERS AT: 4" OC FOR P3 AND P4 SHEAR WALLS 6" OC FOR ALL OTHER SHEAR WALLS		
(4) FOR CONCRETE SPALLS LESS THAN 4", THERE IS NO LOAD REDUCTION AND NO FURTHER REVIEW BY EOR IS REQUIRED.		
(5) SEE DETAIL 4/SD-2 FOR ADD'L CRITERIA AT UPPER FLOOR STRAPS (WHERE OCCURS).		
(6) EDGE NAIL SHT'G TO EA MEMBER OF MULTIPLE POST, OFFSET 1/2 SPACING BTWN MEMBERS.		
(7) ASTM F1554-55 BOLT W/ HEAVY SQUARE NUT OR 1/4 X 1 3/4 X 1 3/4 PLATE WASHER REQUIRED FOR FULL LOAD. REDUCE ALLOWABLE LOAD TO 13160 LBS FOR ASTM GRADE 36 BOLT. MINIMUM EMBEDMENT IS FROM TOP OF FOOTING.		
(8) ASTM GRADE 36 BOLT W/ SQUARE OR HEAVY HEX HEAD OR NUT REQUIRED. MINIMUM EMBEDMENT IS FROM TOP OF FOOTING.		
(9) PROVIDE 6X8 POST AT 2X6 WALLS, MULTIPLE STUDS NOT ALLOWED.		
(10) END POST TO BE FULL HEIGHT MEMBERS, UNO.		
(11) STRAPS MAY BE PLACED ON EITHER FACE OF DESIGNATED WALL AND ARE NOT REQUIRED TO OCCUR ON SAME FACE AS SHEATHING, UNO.		
(12) AT GARAGE STEMWALL LOCATIONS USE SSTBL.		
(13) WHEN SLAB AND FOOTINGS ARE PLACED AS A MONO-POUR, SSTB26 WITH 25" EMBEDMENT MAY BE SUBSTITUTED FOR THE SSTB34 SPECIFIED.		

FOUNDATION NOTES:

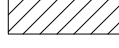
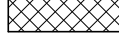
- A. AT INTERIOR BEARING WALLS, WITHOUT DEEPEENED FOOTINGS, USE 1/2" DIA TITEN HD HIGH STRENGTH SCREW ANCHORS IN LIEU OF ANCHOR BOLTS.
- B. SEE DETAIL 1/SD-1 FOR PERIMETER FOOTING EMBEDMENT DEPTH.
- C. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

FRAMING NOTES:

1. ALL EXTERIOR WALLS TO BE MIN 2x6 AT 16" OC DFL STUD GRADE AND INTERIOR BEARING AND SHEAR WALLS TO BE MIN 2x4 AT 16" O.C. DFL STUD GRADE, UNO. SEE FRAMING PLANS FOR NON-TYPICAL STUD SIZE AND SPACING.

2. TRIMMER / KING STUD SCHEDULE, UNO.

2x4 WALLS	OPENING SPAN (L)	TRIMMERS	KING STUDS
	L < 6'-0"	1	1
	6'-0" ≤ L < 10'-0"	2	2
2x6 WALLS	L < 8'-0"	1	1
	8'-0" ≤ L < 12'-0"	2	2
	12'-0" ≤ L < 20'-0"	2	3

3.  BLOCKED DIAPHRAGM - SEE STRUCTURAL GENERAL NOTES SHEET S1.
4. FOR TYPICAL OVERFILL FRAMING WHERE REQUIRED BY TRUSS SHOP DRAWINGS, SEE DETAILS 4/SD-3 OR 5/SD-3.
5.  INTERIOR BEARING WALLS
6. BEAM AND HEADER SIZES INDICATED ON THIS PLAN ARE MINIMUM. LARGER SIZES OR HIGHER GRADE LUMBER MAY BE SUBSTITUTED.
7. TOP PLATE SPLICES PER DETAIL 3/SD-2, UNO.
8. SEE DETAIL 8/SD-2 FOR ADDITIONAL FRAMING REQUIREMENTS.

LEDGER & HANGER SCHEDULE:

UNLESS OTHERWISE NOTED 2X LEDGERS WHERE DETAILED SHALL BE AS FOLLOWS:

		TRUSS		LEDGER AND NAILING ^(1, 2, 6, 10, 11)	MIN HANGER, UNO ^(3, 4, 5, 11)
		SPAN (L)	SPACING (MAX)		
ROOF LOAD		L ≤ 8'-0"	16" O.C.	2X6 W/ (3) 16d AT 16"OC	LUS24/JUS24
			24" O.C.	2X8 W/ (5) 16d AT 24"OC	LUS24/JUS24
		L ≤ 16'-0"	24" O.C.	2X8 W/ (5) 16d AT 16"OC	LUS26/JUS26
				2X12 W/ (8) 16d AT 24"OC	LUS26/JUS26
FLOOR LOAD	4x MEMBER	L ≤ 10'-0"	16" O.C.	2X8 W/ (5) 16d AT 16"OC	LUS46/JUS46
			24" O.C.	2X10 W/ 2 COLUMNS OF (6) 16d AT 24"OC	LUS46/JUS46
		L ≤ 20'-0"	24" O.C.	2X10 W/ 2 COLUMNS OF (6) 16d AT 16"OC	LUS48/JUS48
			24" O.C.	2X12 W/ 2 COLUMNS OF (8) 16d AT 24"OC	HUS48/HUS48
FLOOR LOAD	2x MEMBER	L ≤ 10'-0"	16" O.C.	2X8 W/ (5) 16d AT 16"OC	LUS26/JUS26
			24" O.C.	2X10 W/ 2 COLUMNS OF (6) 16d AT 24"OC	LUS26/JUS26
		L ≤ 20'-0"	24" O.C.	2X10 W/ 2 COLUMNS OF (6) 16d AT 16"OC	LUS28/JUS28
			24" O.C.	2X12 W/ 2 COLUMNS OF (8) 16d AT 24"OC	HUS28/HUS28
DECK LOAD	2x MEMBER	L ≤ 10'-0"	16" O.C.	2X10 W/ (2) 1/4"x3" SCREWS AT 16" OC	LUS26/JUS26
			24" O.C.	2X10 W/ 2 COLUMNS OF (2) 1/4"x3" SCREWS AT 24" OC	LUS26/JUS26
		L ≤ 15'-0"	24" O.C.	2X10 W/ 2 COLUMNS OF (2) 1/4"x3" SCREWS AT 16" OC	LUS28/JUS28
			24" O.C.	2X10 W/ 2 COLUMNS OF (3) 1/4"x3" SCREWS AT 24" OC	HUS28/HUS28

SCHEDULE NOTES:

- (1) TWO COLUMNS OF FASTENERS REQUIRE MIN SUPPORTING MEMBER RECEIVING FASTENERS OF 3" OR (2)2X IN WIDTH (ENDS OF 4X2 OR DOUBLE TRUSSES ARE ACCEPTABLE). SPACE FASTENERS MIN 1" APART IN EACH DIRECTION.
- (2) SPACING SHOWN EQUALS SPACING OF FRAMING MEMBERS RECEIVING FASTENERS.
- (3) HANGERS LISTED IN ORDER ARE BY SIMPSON STRONG-TIE AND USP, RESPECTIVELY.
- (4) LISTED HANGERS ARE MINIMUM HANGERS REQUIRED WHERE NOT OTHERWISE NOTED ON THE STRUCTURAL DRAWINGS.
- (5) SPACING SHOWN FOR HANGERS IS SPACING OF THE FRAMING MEMBER SUPPORTED BY THE HANGER.
- (6) LEDGER MATERIAL SHALL BE DFL #2 OR BETTER UNO. LEDGER AT DECK SHALL BE PRESSURE-PRESERVATIVE TREATED OR NATURALLY DURABLE WOOD.
- (7) PROVIDE THA218 (MIN) HANGER FOR ROOF TRUSS TO BEAM CONNECTIONS FOR ALL OVERHEAD - 08/17/2021
- (8) PROVIDE THA418 (MIN) HANGER FOR FLOOR TRUSS TO BEAM CONNECTIONS FOR ALL BEAM DEPTHS GREATER THAN 10 INCHES.
- (9) PROVIDE THA422 (MIN) HANGER FOR FLOOR TRUSS TO BEAM CONNECTIONS FOR ALL BEAM DEPTHS GREATER THAN 18 INCHES.
- (10) ALL SIMPSON SCREWS NOTED SHALL BE STRONG DRIVE SDS SCREWS. ALL USP SCREWS NOTED SHALL BE WS SCREWS.
- (11) HANGERS AND FASTENERS WITH EXTERIOR EXPOSURE SHALL BE STAINLESS STEEL, UNLESS NOTED OTHERWISE.

JOB NO: 1939-006-201
DESIGNED BY: SLB
DRAWN BY: ACM
ISSUED FOR: CONSTRUCTION DOCUMENTS
DATE: 07-09-20
SHEET TITLE: SCHEDULES AND NOTES
REVISIONS:

L. R. NELSON CONSULTING ENGINEERS, LLC.

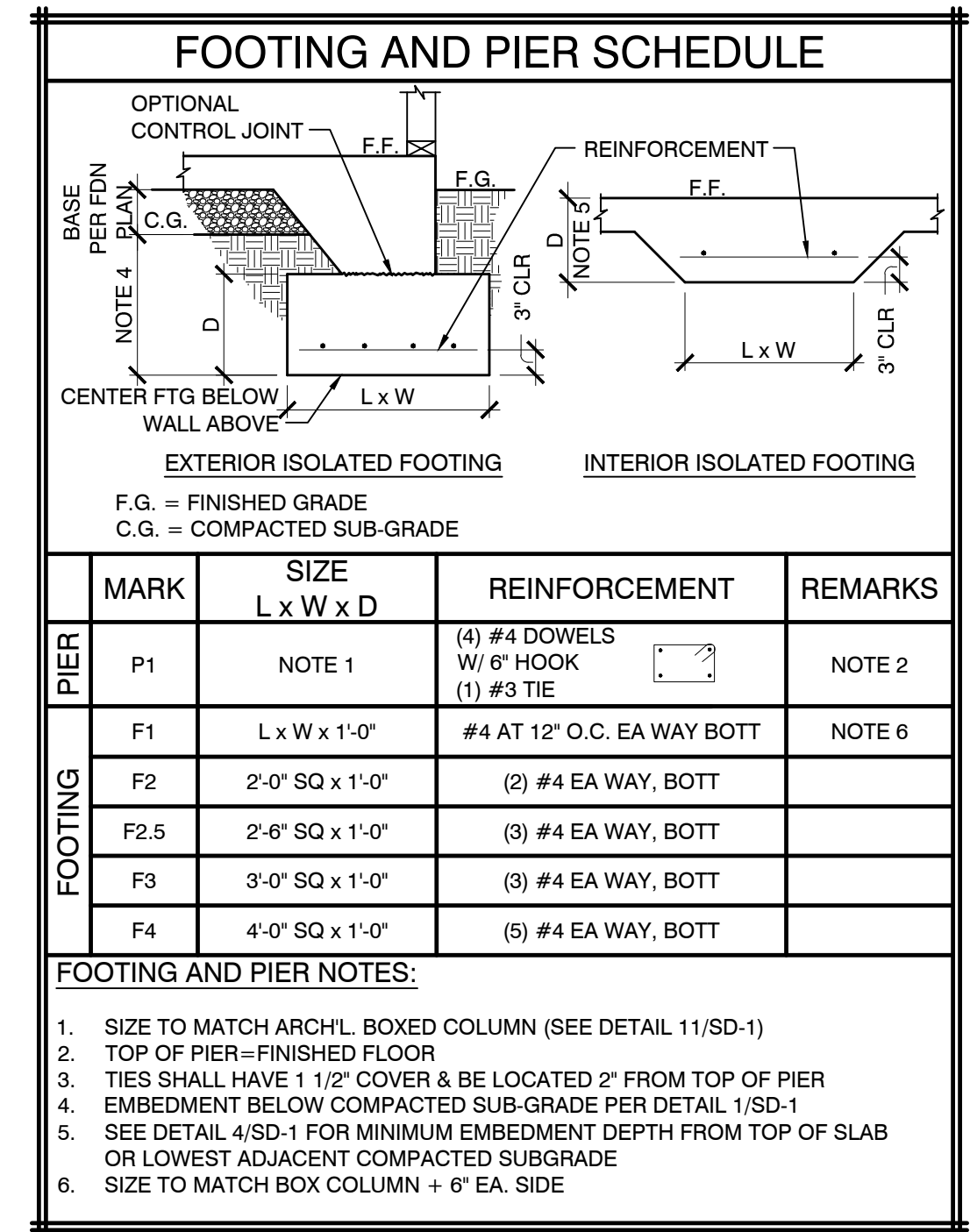
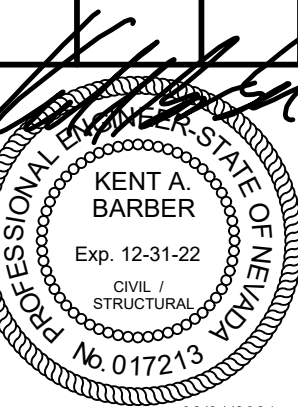
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• SURVEY

ASSURED DEVELOPMENT
736 NAPLES AVENUE
HENDERSON, NV

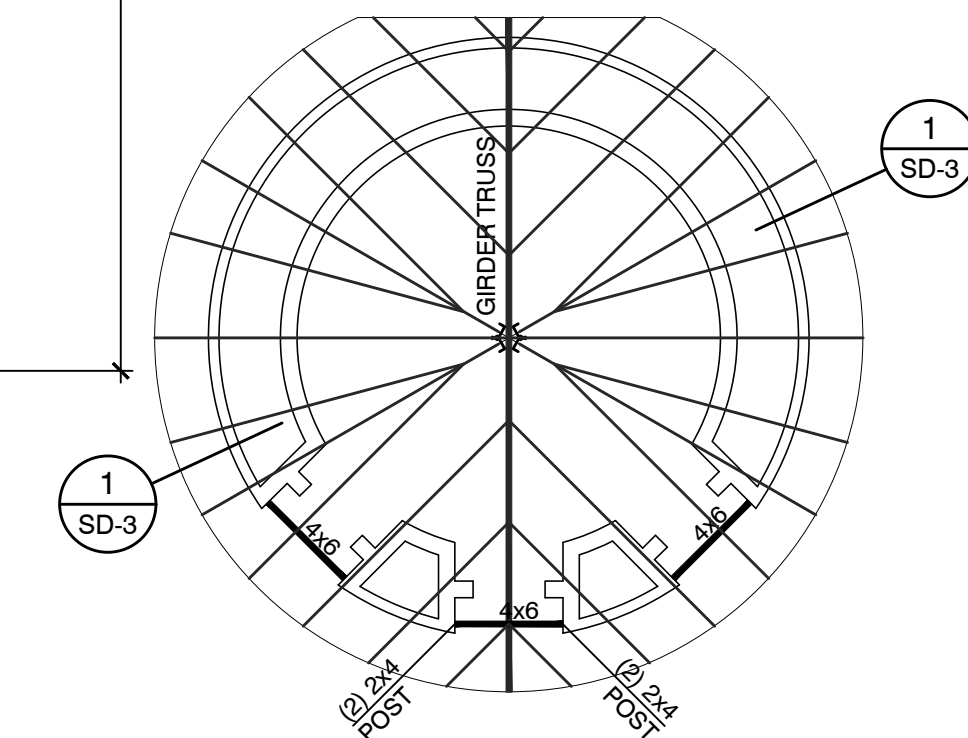
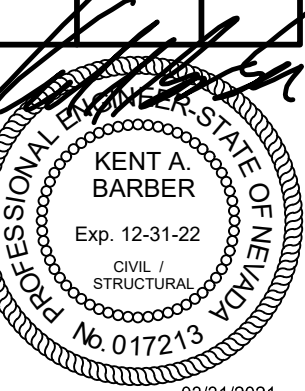


S1.1


$$3/16'' = 1'-0''$$

BY PAUL LUM
INTERWEST CONSULTING GROUP

SEE SHEET S1.1 FOR FOUNDATION
NOTES AND HOLDOWN SCHEDULE


$$3/16'' = 1'-0''$$


ASSURED DEVELOPMENT
736 NAPLES AVENUE
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S3

$$3/16'' = 1' - 0'$$

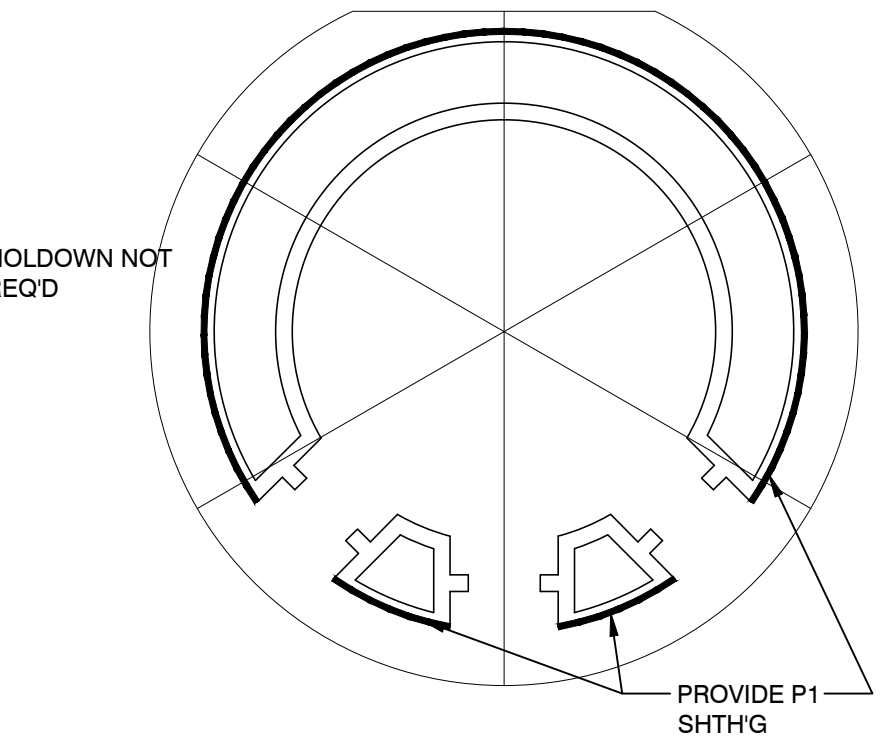
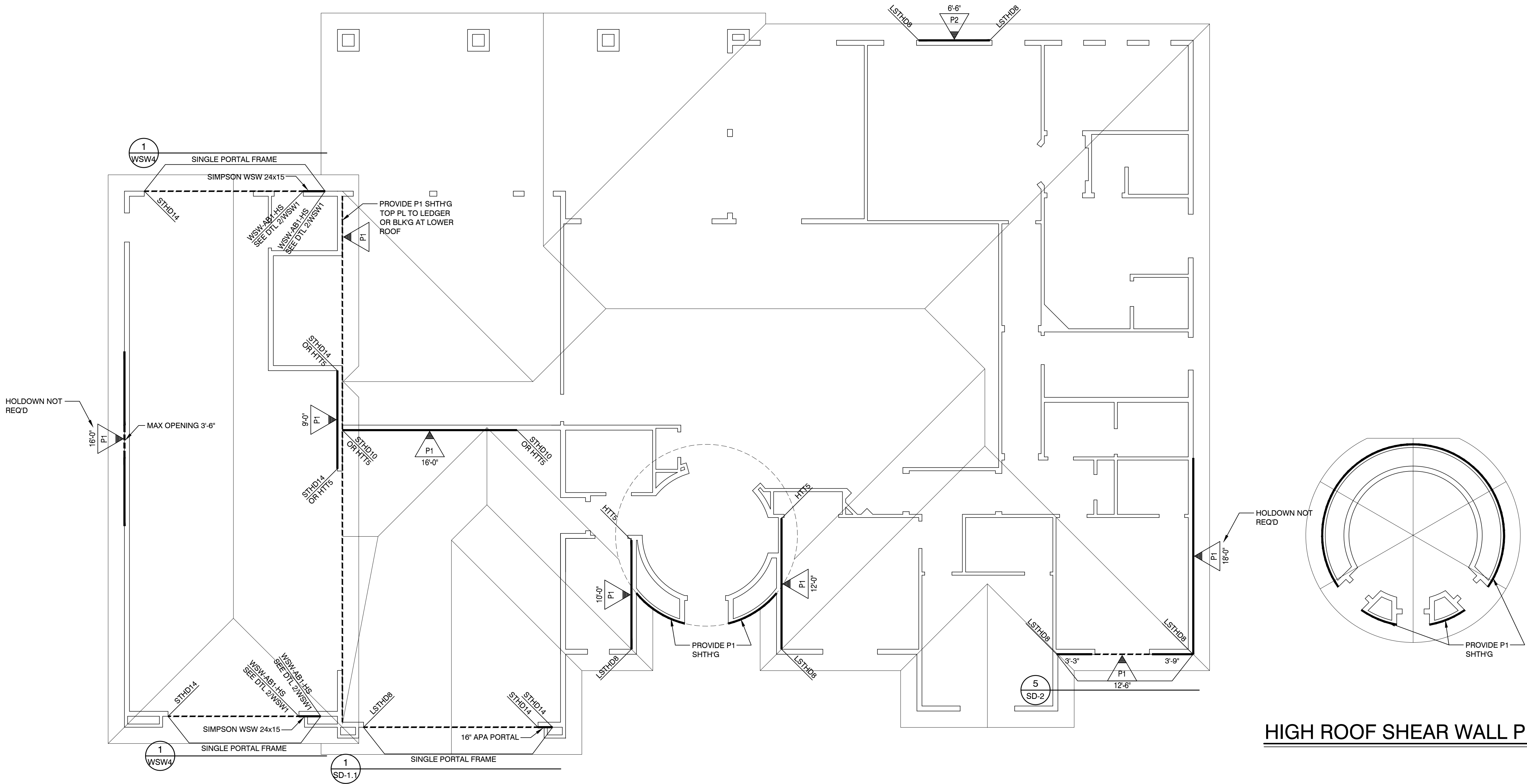
BUILDING CODE COMPLIANCE REVIEW
THIS SET OF PLANS AND SPECIFICATIONS HAVE BEEN REVIEWED
FOR COMPLIANCE WITH THE LOCALLY ADOPTED CODES RELATE
TO THIS PROJECTS SCOPE OF WORK

Jul 23, 2021

BY PAUL LUM
INTERWEST CONSULTING GROUP

SEE SHEET S1.1 FOR FRAMING NOTES

JOB NO: 1939-006-201
DESIGNED BY: SLB
DRAWN BY: ACM
ISSUED FOR: CONSTRUCTION DOCUMENTS
DATE: 07-09-20
SHEET TITLE: SHEAR WALL PLAN
REVISIONS:



HIGH ROOF SHEAR WALL PLAN

3/16" = 1'-0"

SHEAR WALL PLAN

3/16" = 1'-0"



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PROFESSIONAL ENGINEER STATE OF NEVADA

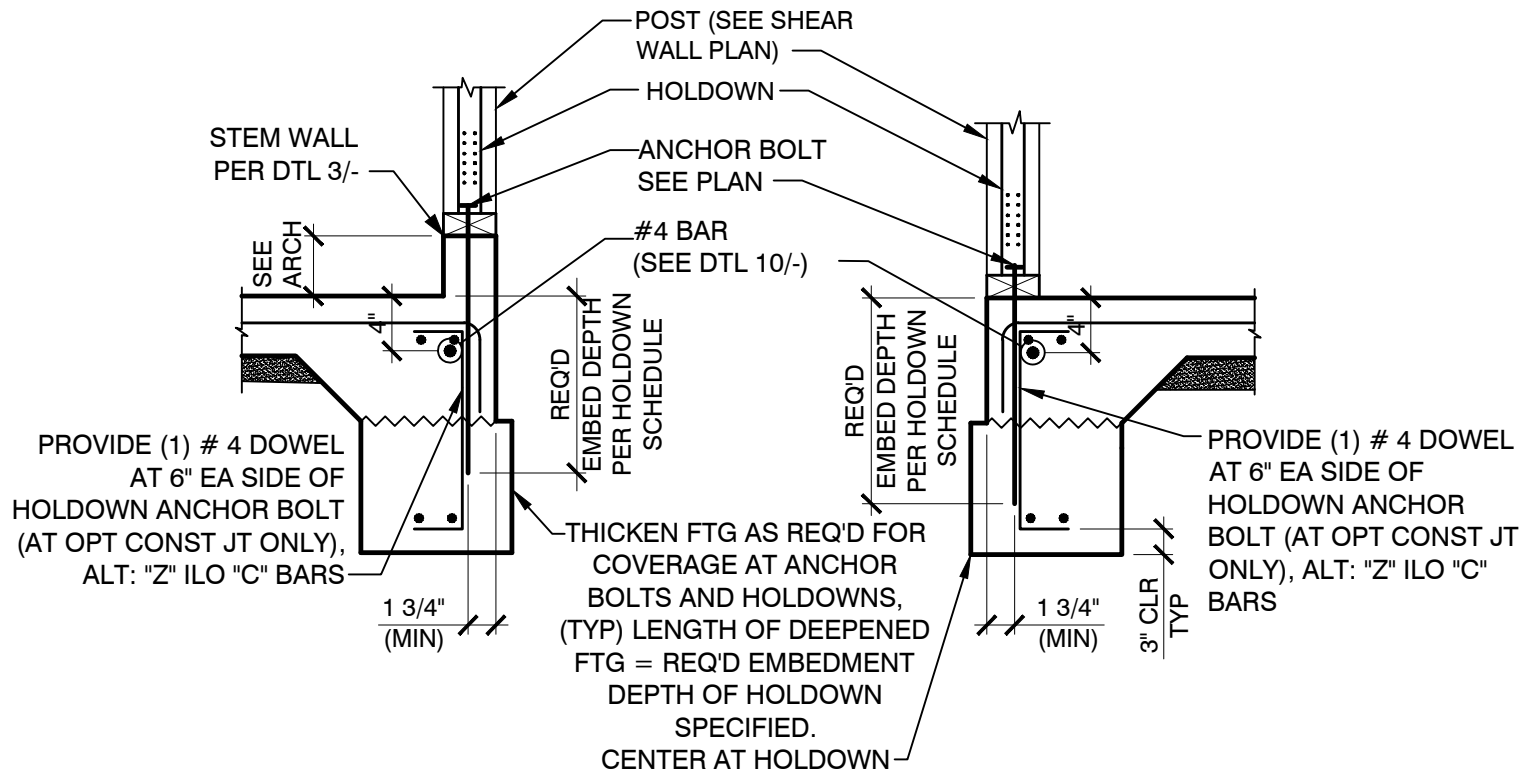
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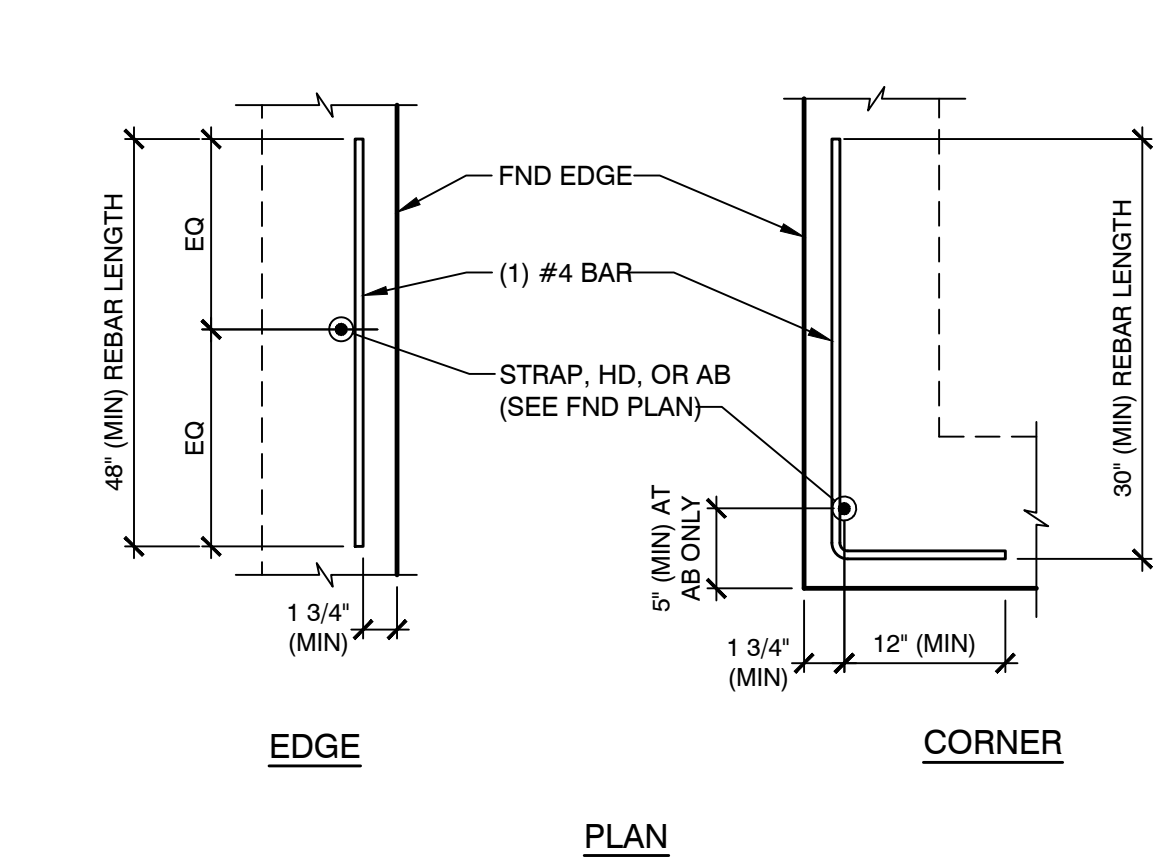
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S4

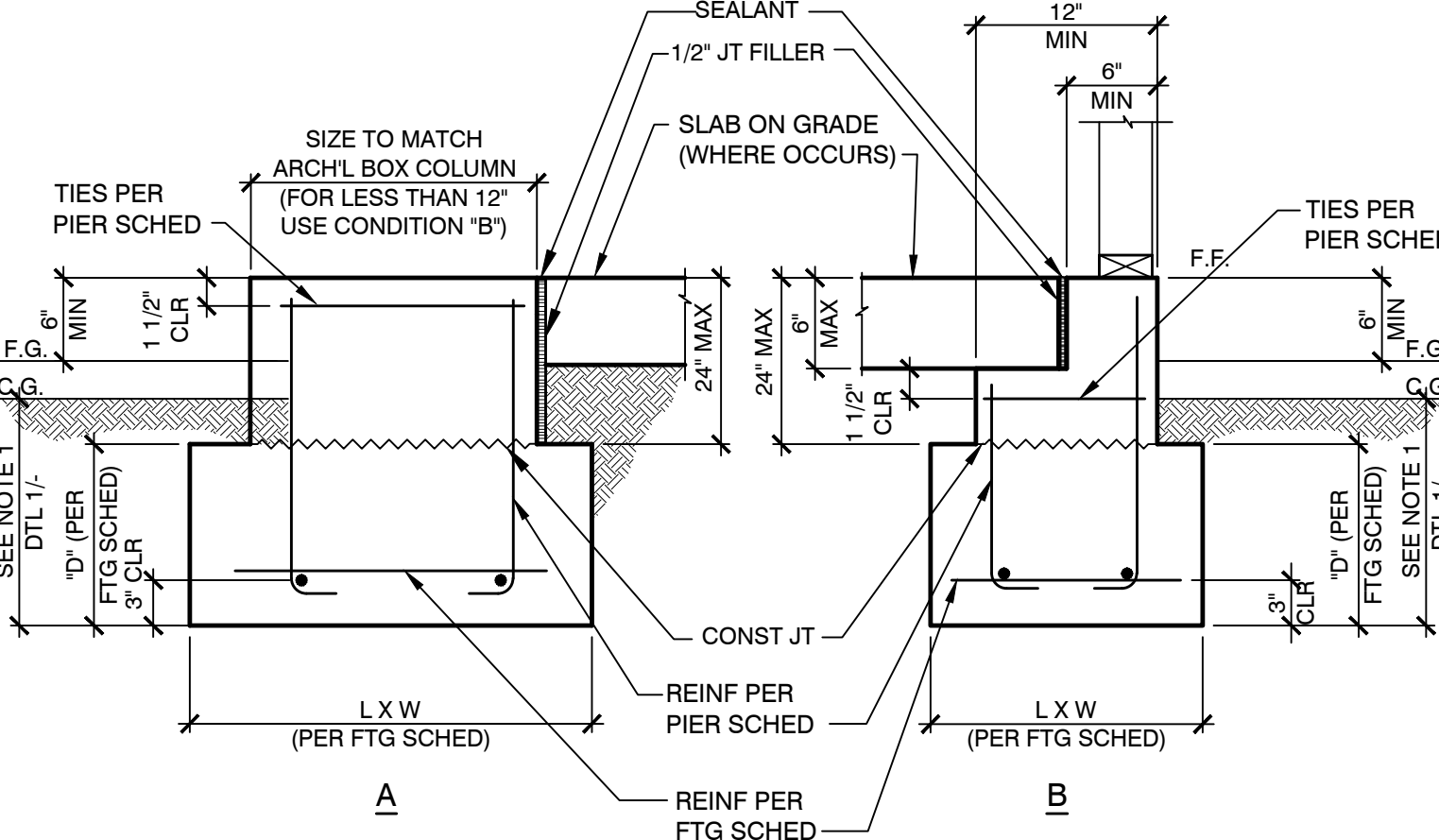
SEE SHEET S1.1 FOR SHEAR WALL
AND HOLDOWN SCHEDULES



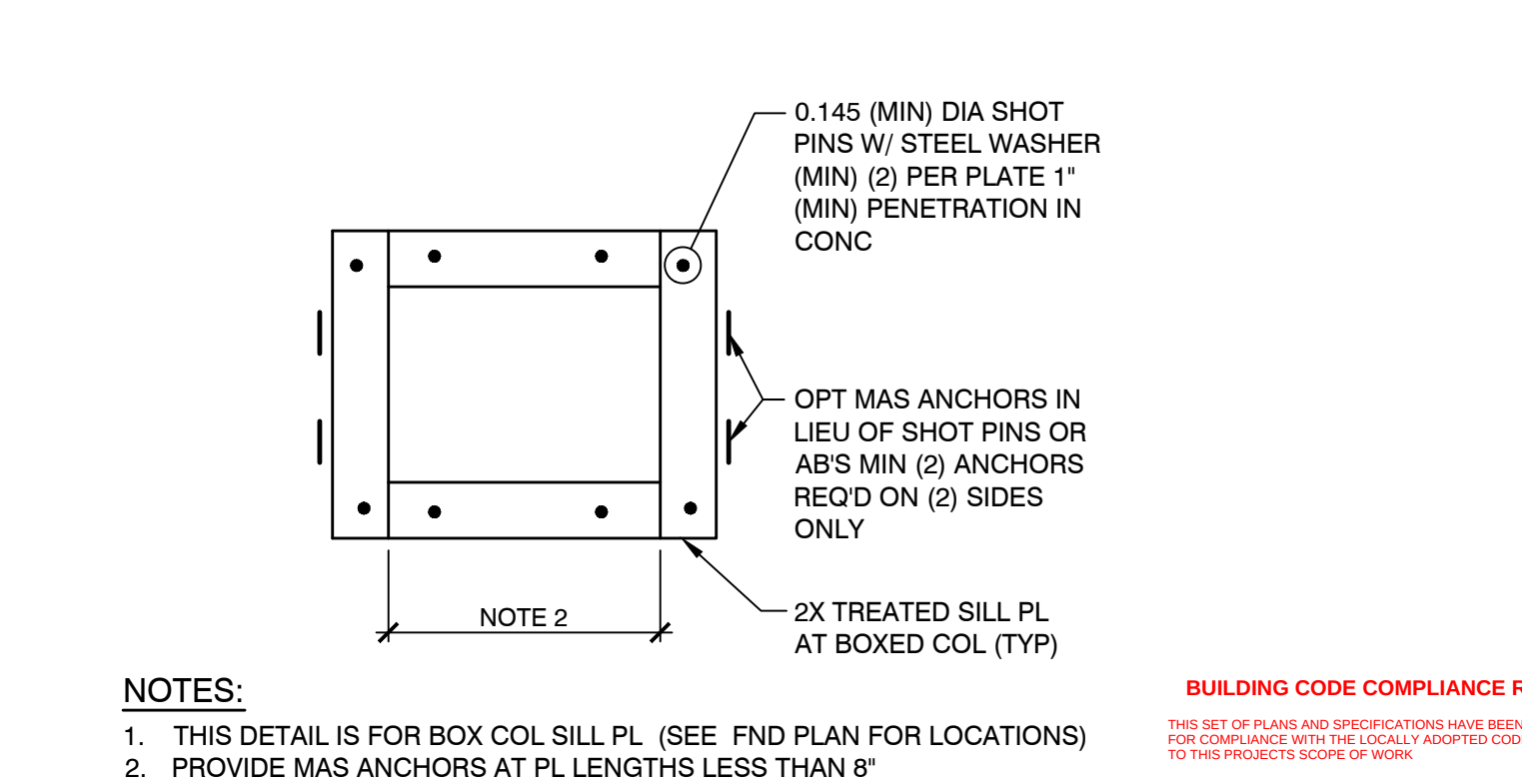
9 HOLDOWN W/ ANCHOR BOLT (TYPICAL) NTS



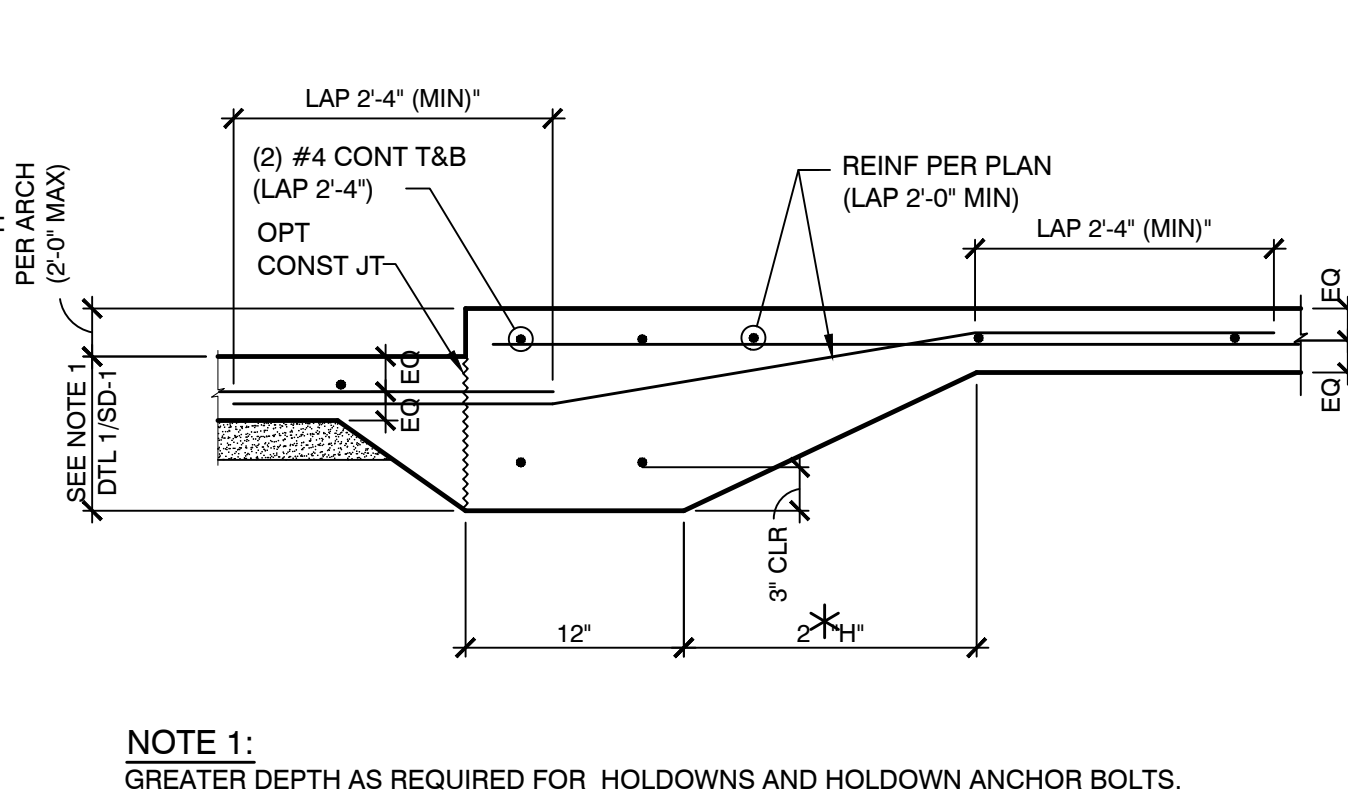
10 REINFORCING AT HOLDOWNS NTS



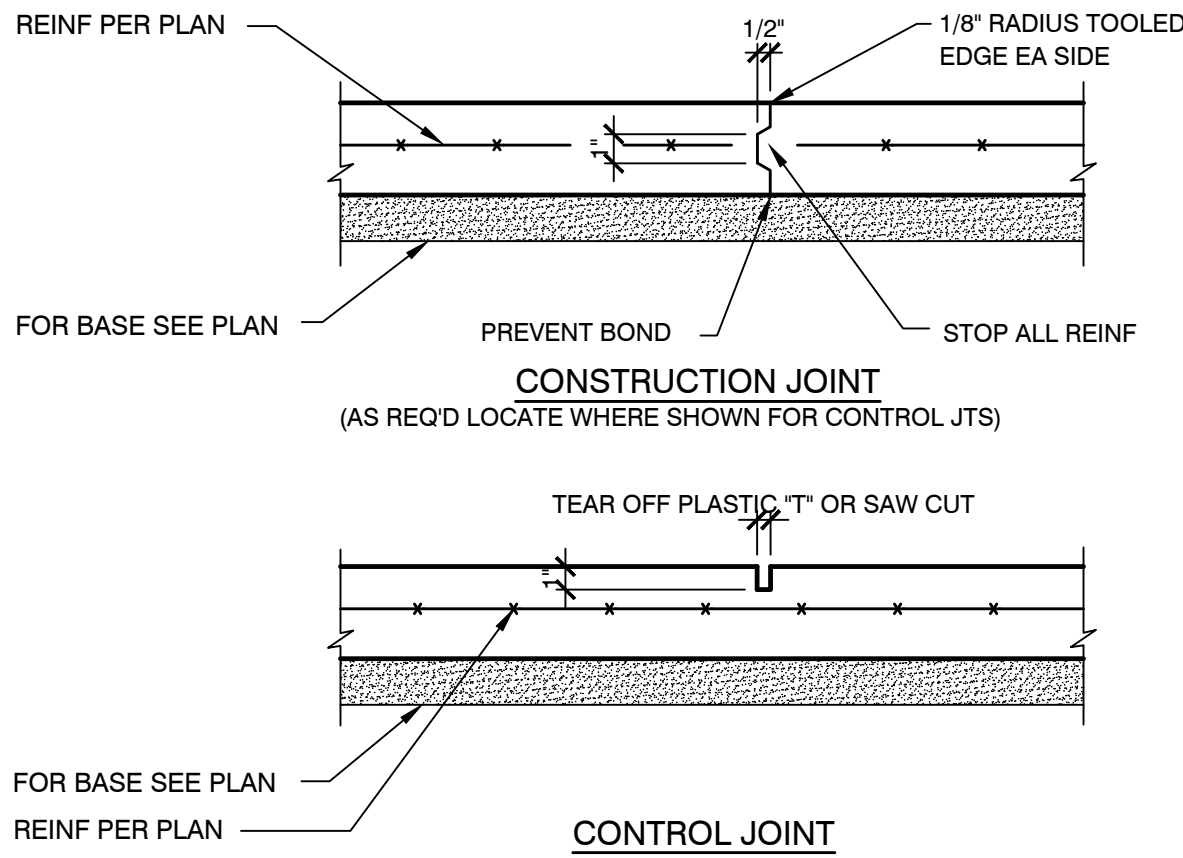
11 CONCRETE PIER AND FOOTING NTS



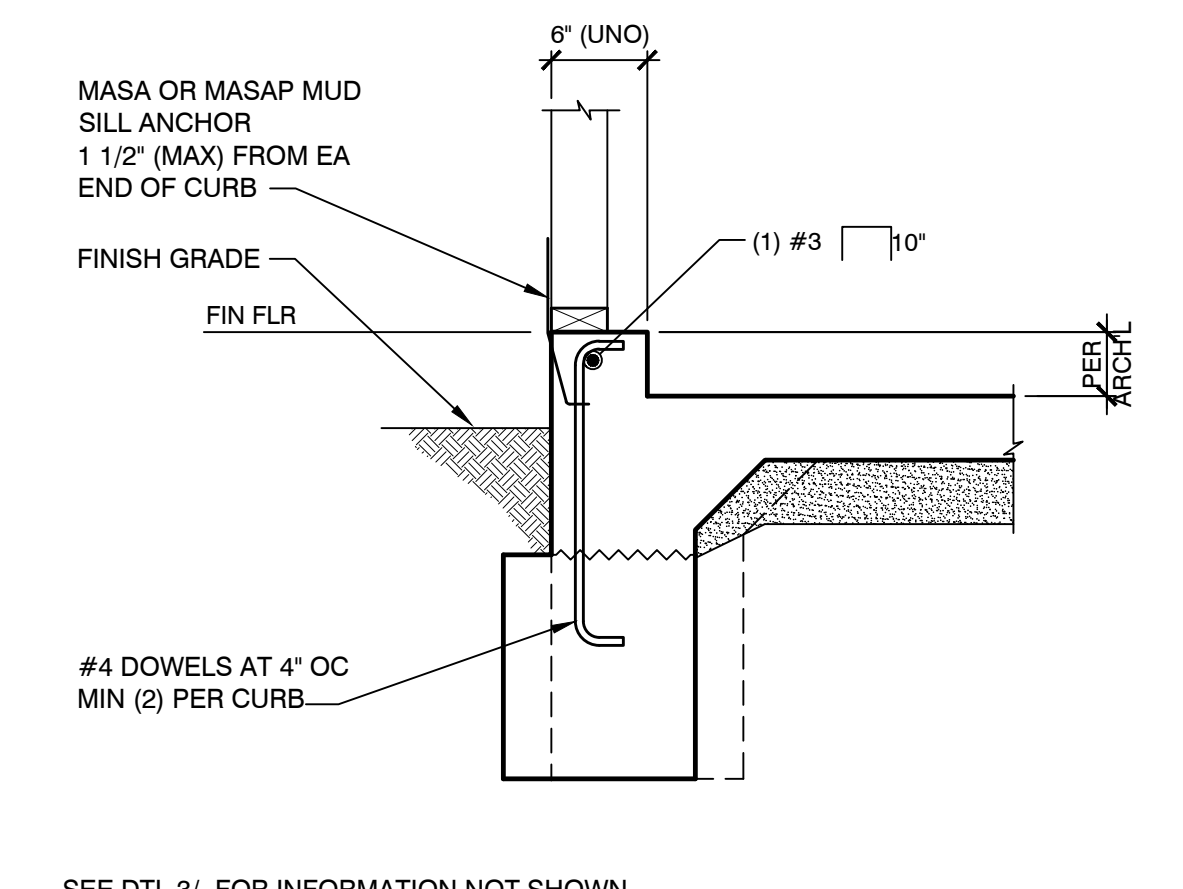
12 BOX COLUMN SILL PLATE NTS



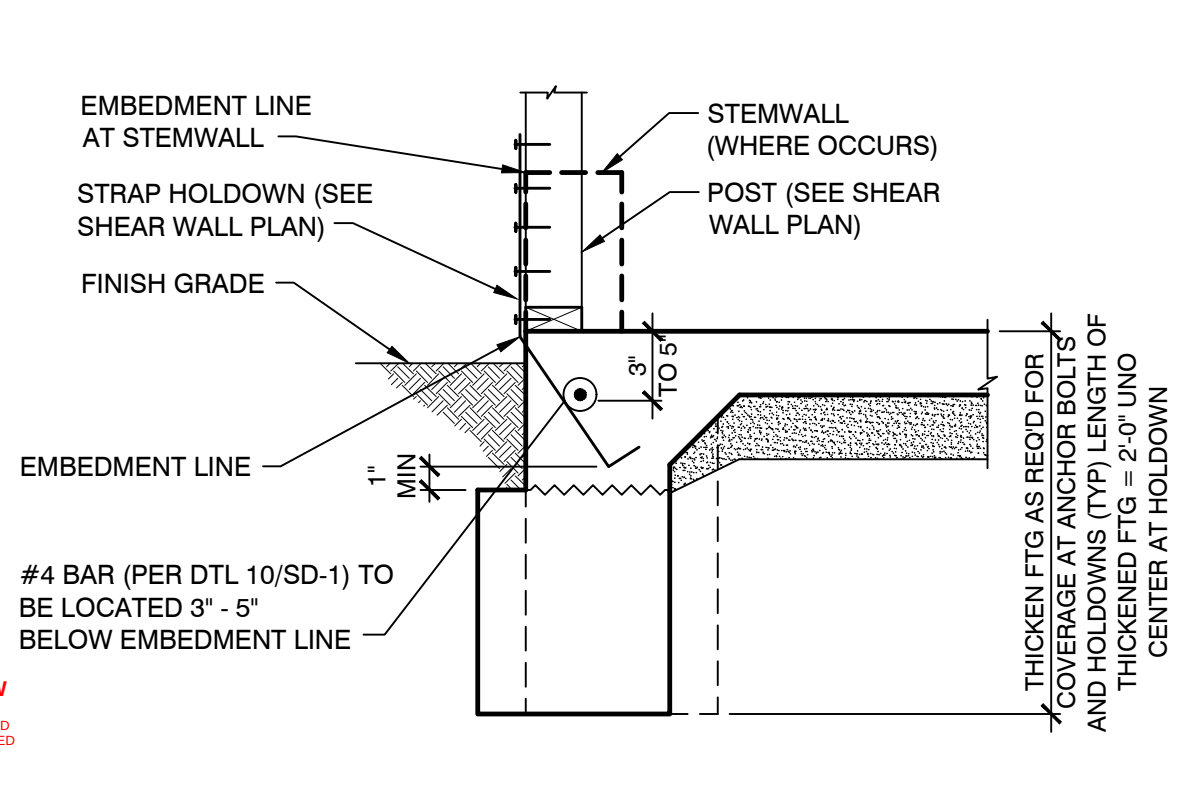
5 STEP IN SLAB NTS



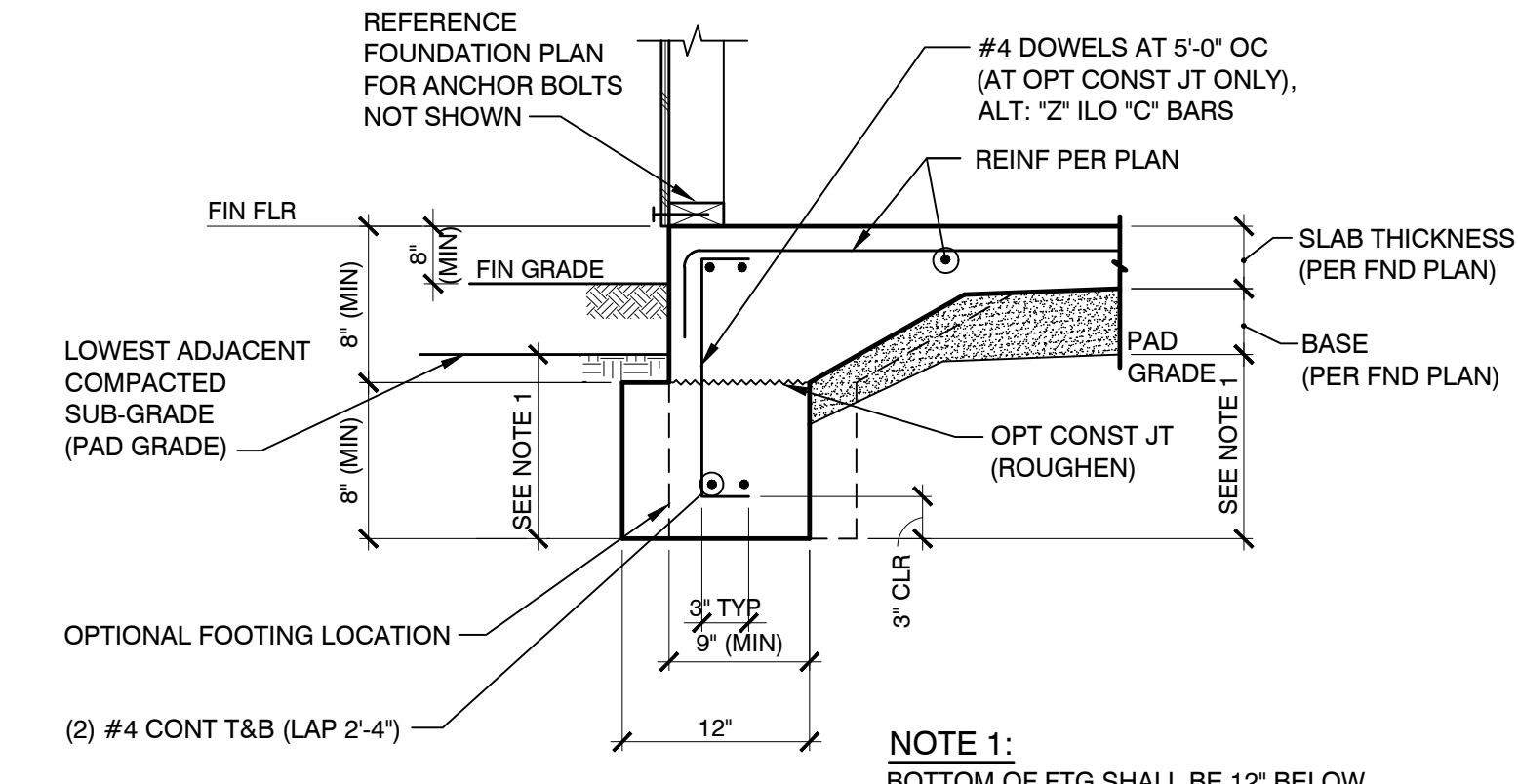
6 TYP SLAB JOINTS NTS



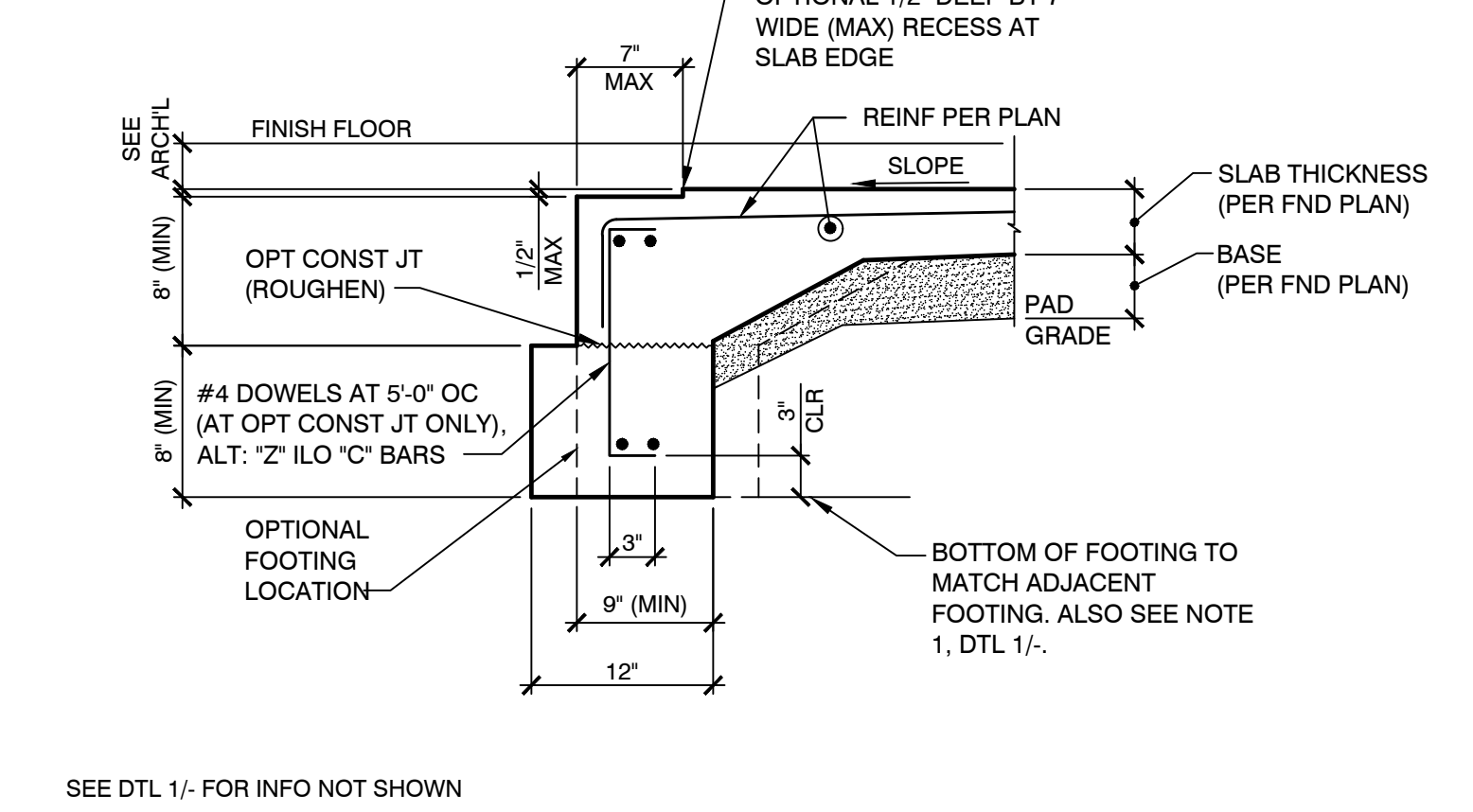
7 CURB BETWEEN GARAGE DOORS NTS



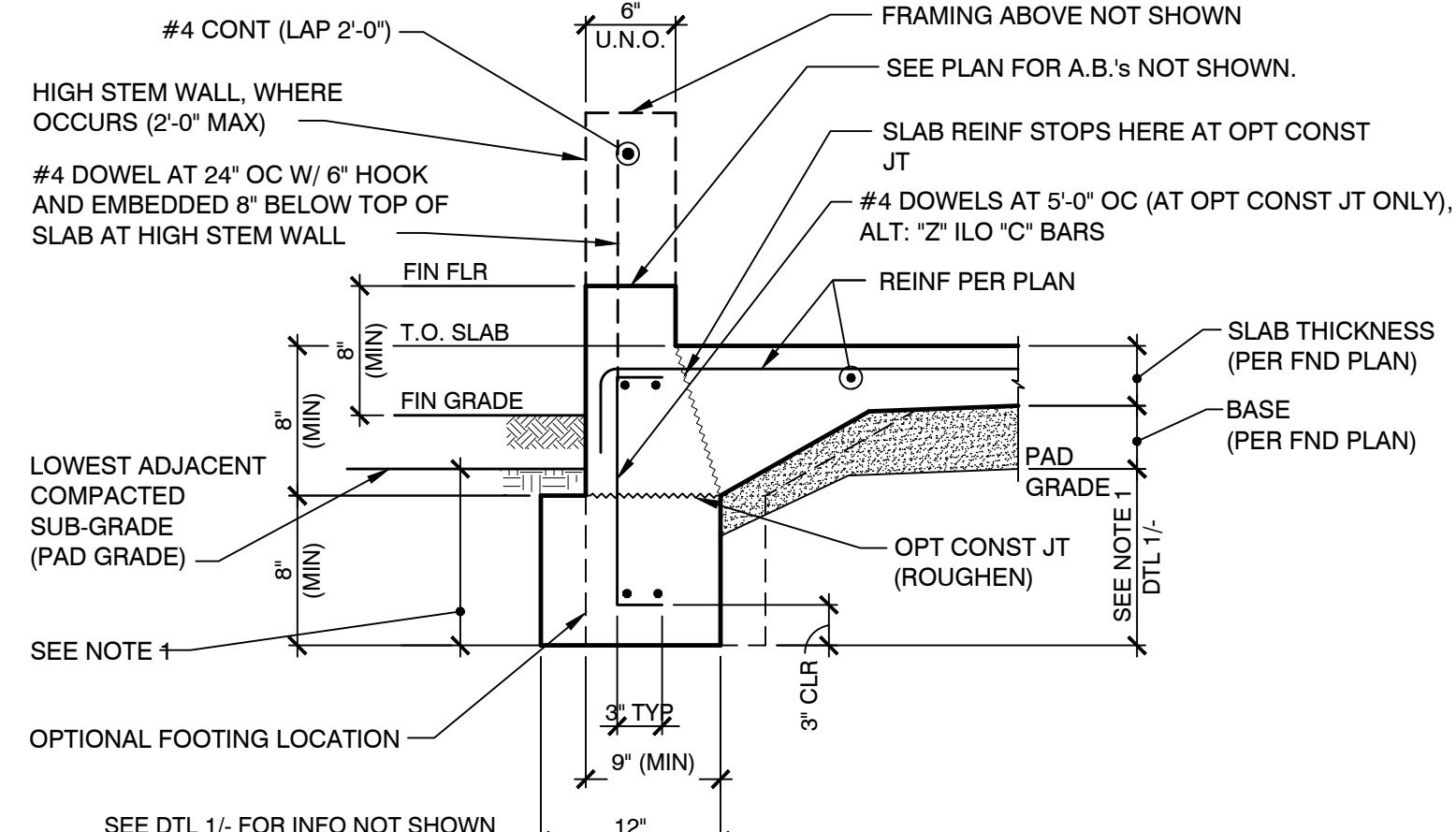
8 STRAP HOLDOWN NTS



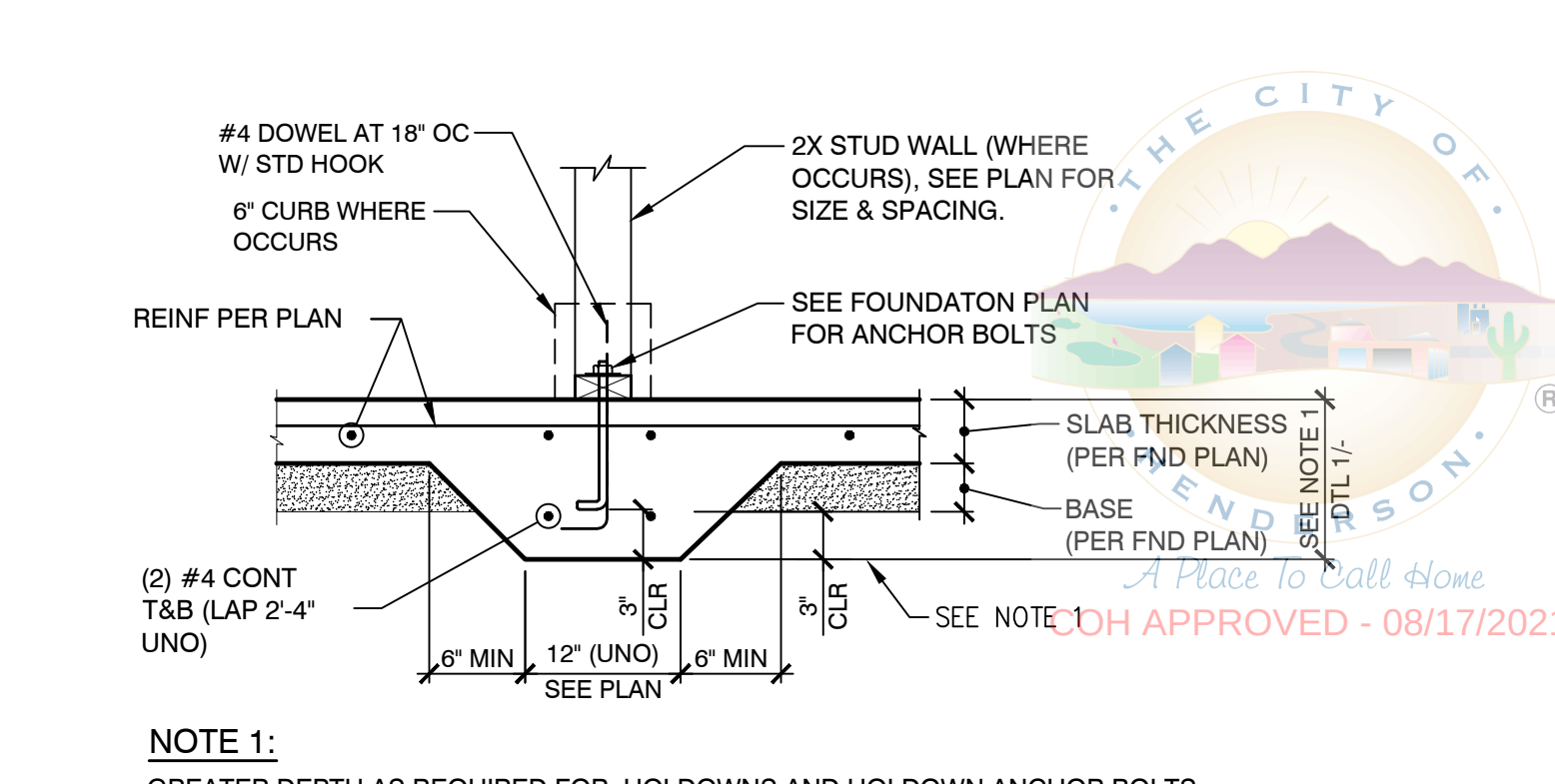
1 EDGE OF FOUNDATION NTS



2 EDGE OF FOUNDATION (GARAGE) NTS



3 EDGE OF FOUNDATION (CURB) NTS



4 INTERIOR CONTINUOUS FTG NTS

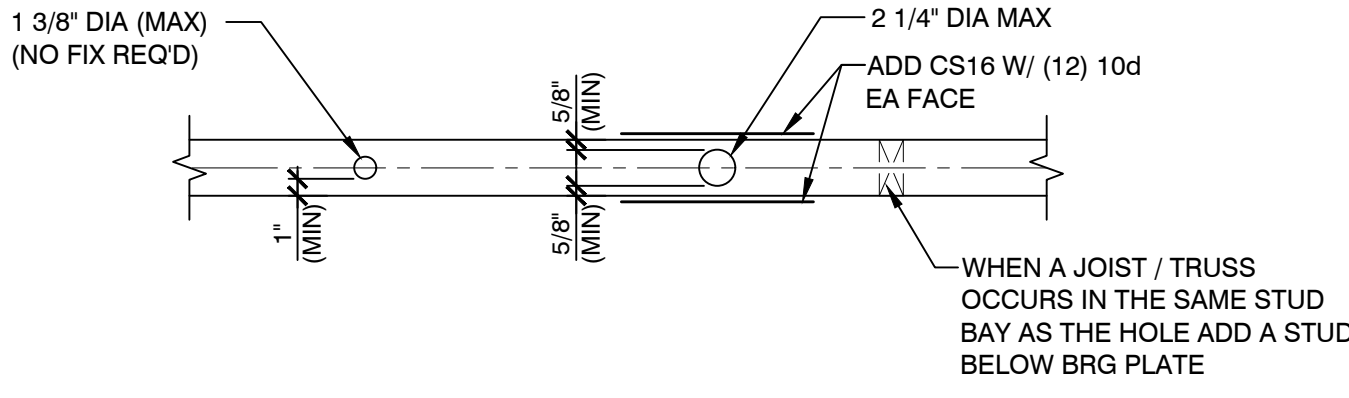
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ISSUED FOR: CONSTRUCTION DOCUMENTS
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SHEET TITLE: STRUCTURAL DETAILS
REVISIONS:

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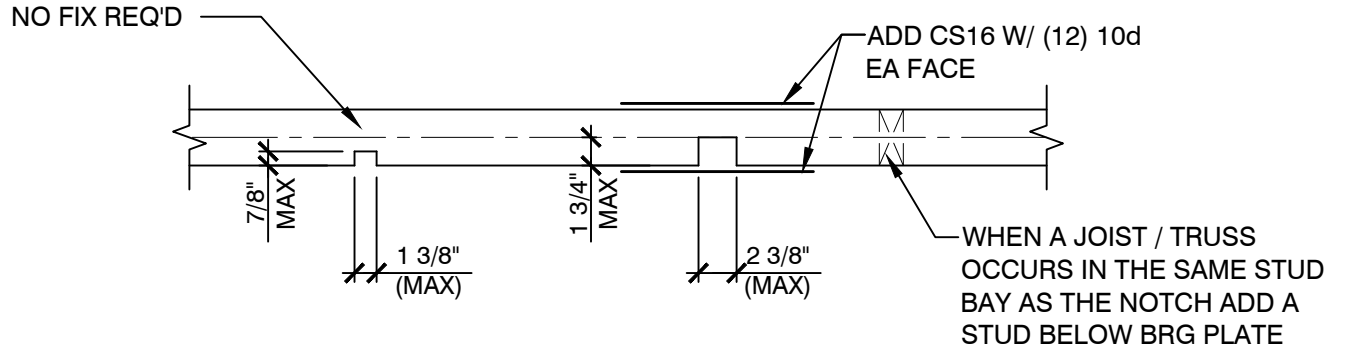
ASSURED DEVELOPMENT
736 NAPLES AVENUE
HENDERSON, NV

KENT A. BARBER
Exp. 12-31-22
No. 017213
03/31/2021

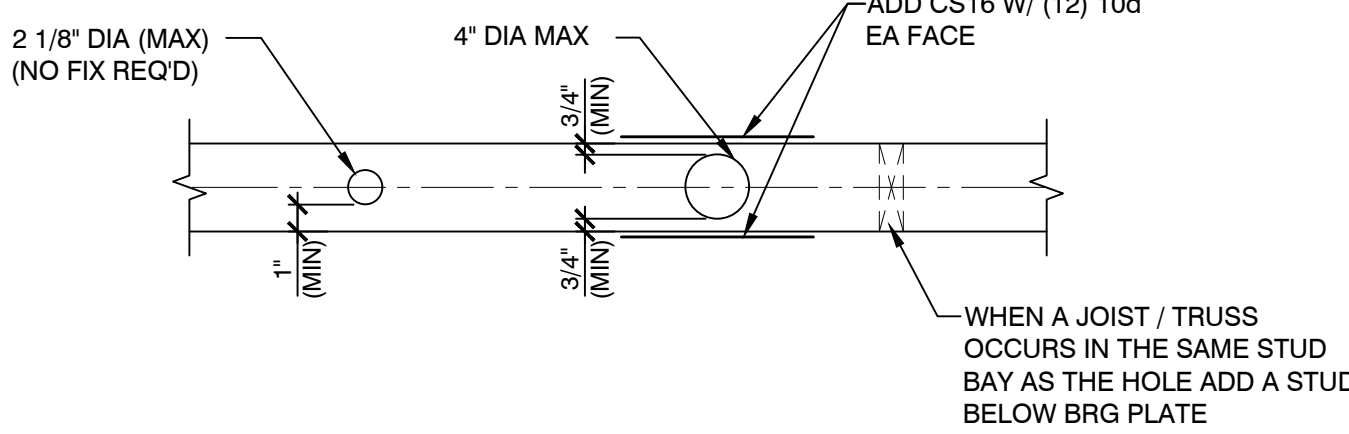
SD-1



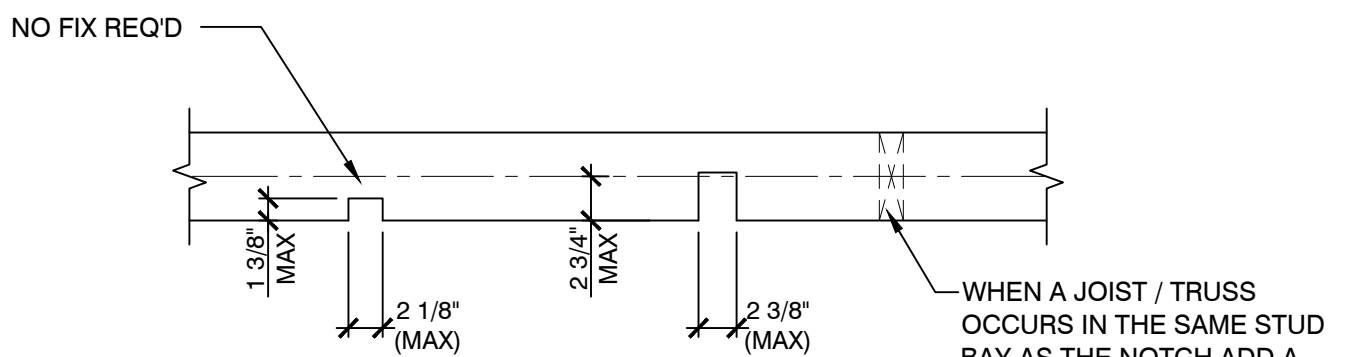
2X4 PLATE W/ DRILLED HOLES



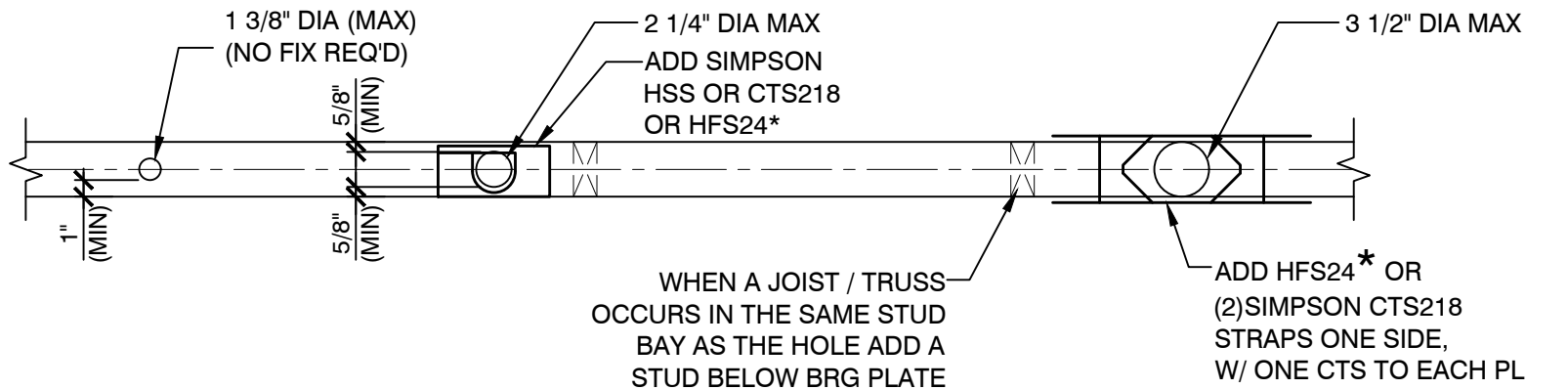
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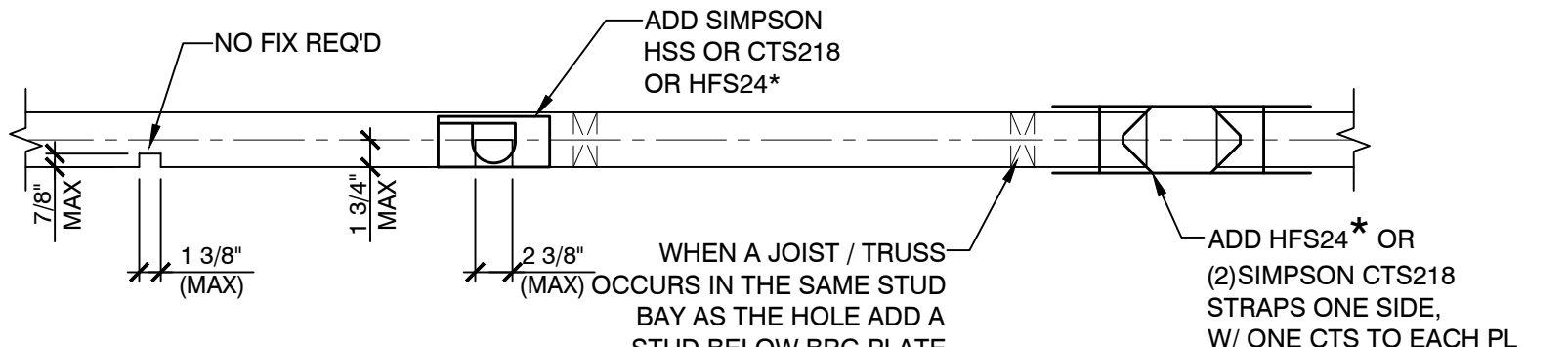
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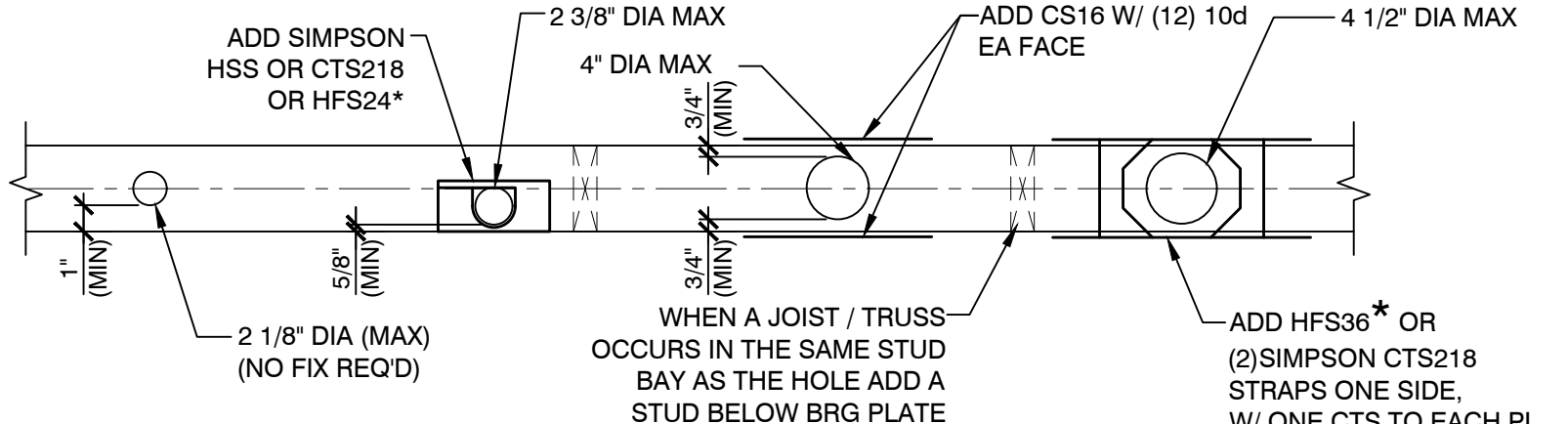
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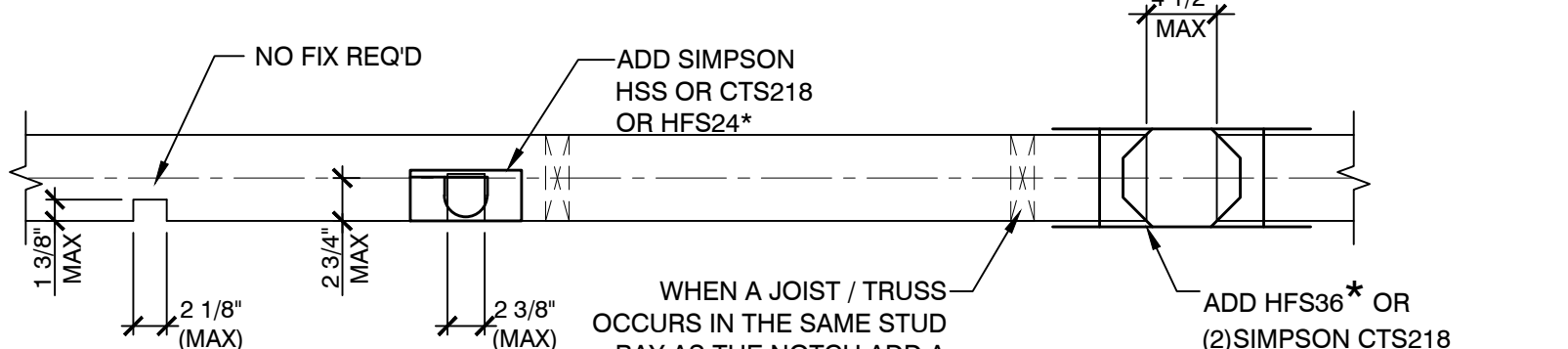
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2X4 PLATE W/ NOTCHES



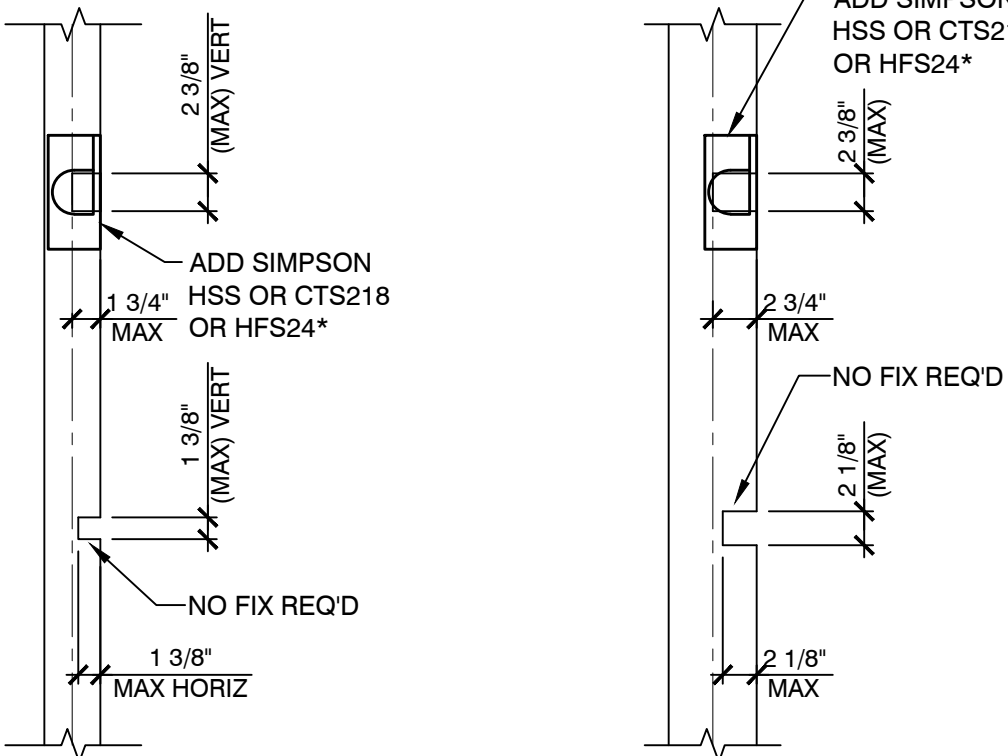
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2X6 PLATE W/ NOTCHES

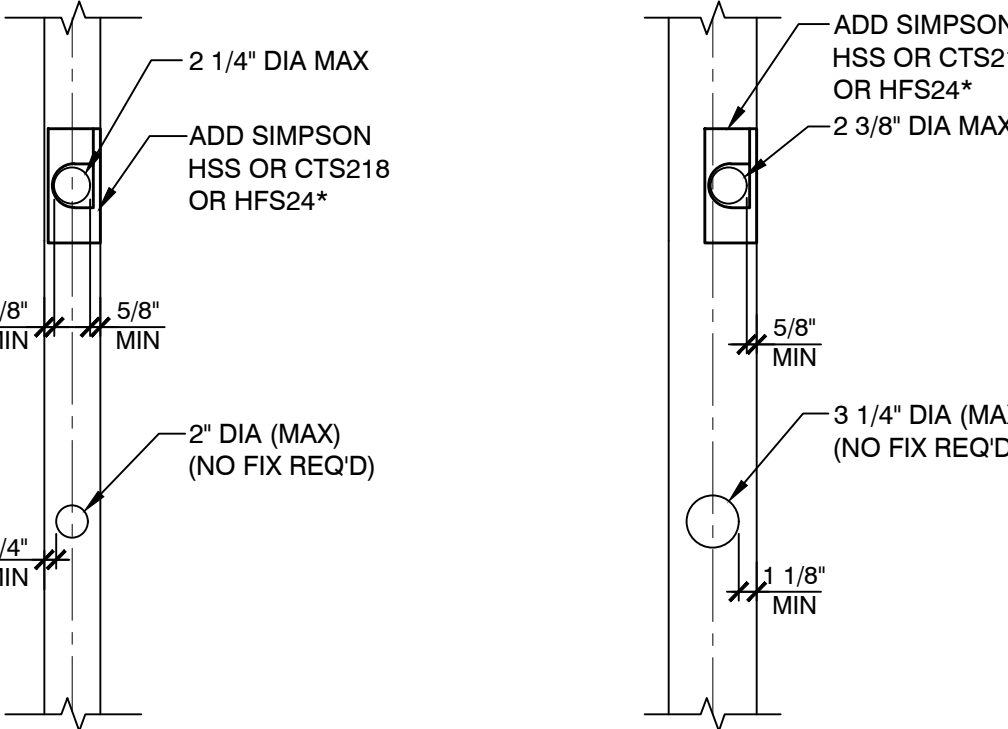
* HARDWARE MANUFACTURED BY HARDY
FRAMES SEE IMFR PRODUCT CATALOG

3 ALLOWABLE HOLES AND NOTCHES IN PLATES
INTERIOR WALLS (BEARING AND NON-BEARING) NTS



2X4 STUD W/ NOTCHES

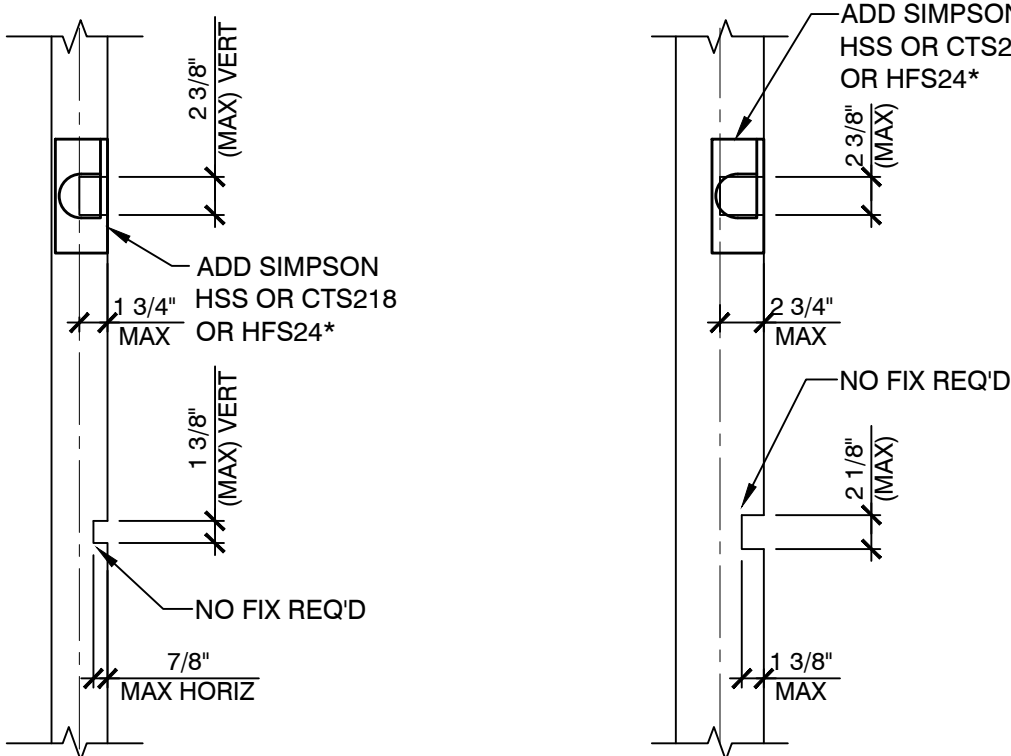
2X6 STUD W/ NOTCHES



2X4 STUD W/ DRILLED HOLES 2X6 STUD W/ DRILLED HOLES

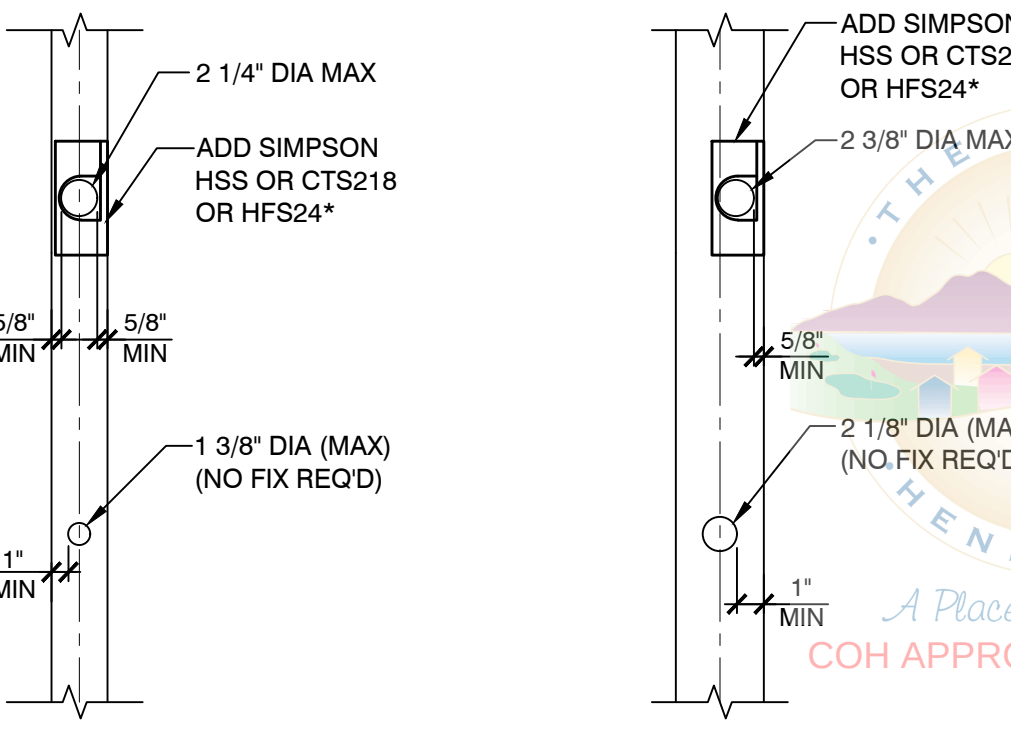
4 ALLOWABLE HOLES AND NOTCHES IN STUDS
INTERIOR NON-BEARING WALLS NTS

1 ALLOWABLE HOLES AND NOTCHES IN PLATES
EXTERIOR WALLS AND INTERIOR SHEAR WALLS NTS



2X4 STUD W/ NOTCHES

2X6 STUD W/ NOTCHES



2X4 STUD W/ DRILLED HOLES 2X6 STUD W/ DRILLED HOLES

2 ALLOWABLE HOLES AND NOTCHES IN STUDS
EXTERIOR WALLS, INTERIOR SHEAR WALLS AND INTERIOR BEARING WALLS NTS

BUILDING CODE COMPLIANCE REVIEW
THIS SET OF PLANS AND SPECIFICATIONS HAVE BEEN REVIEWED
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Jul 23, 2021

BY PAUL LUM
INTERWEST CONSULTING GROUP

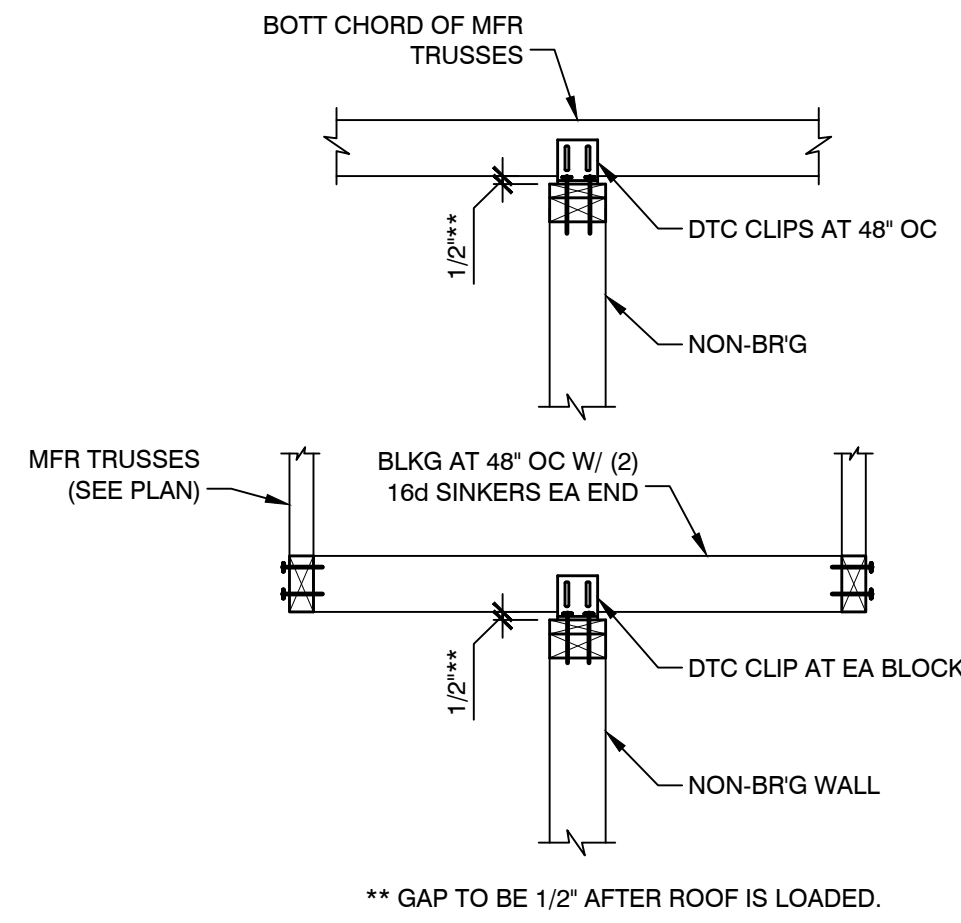
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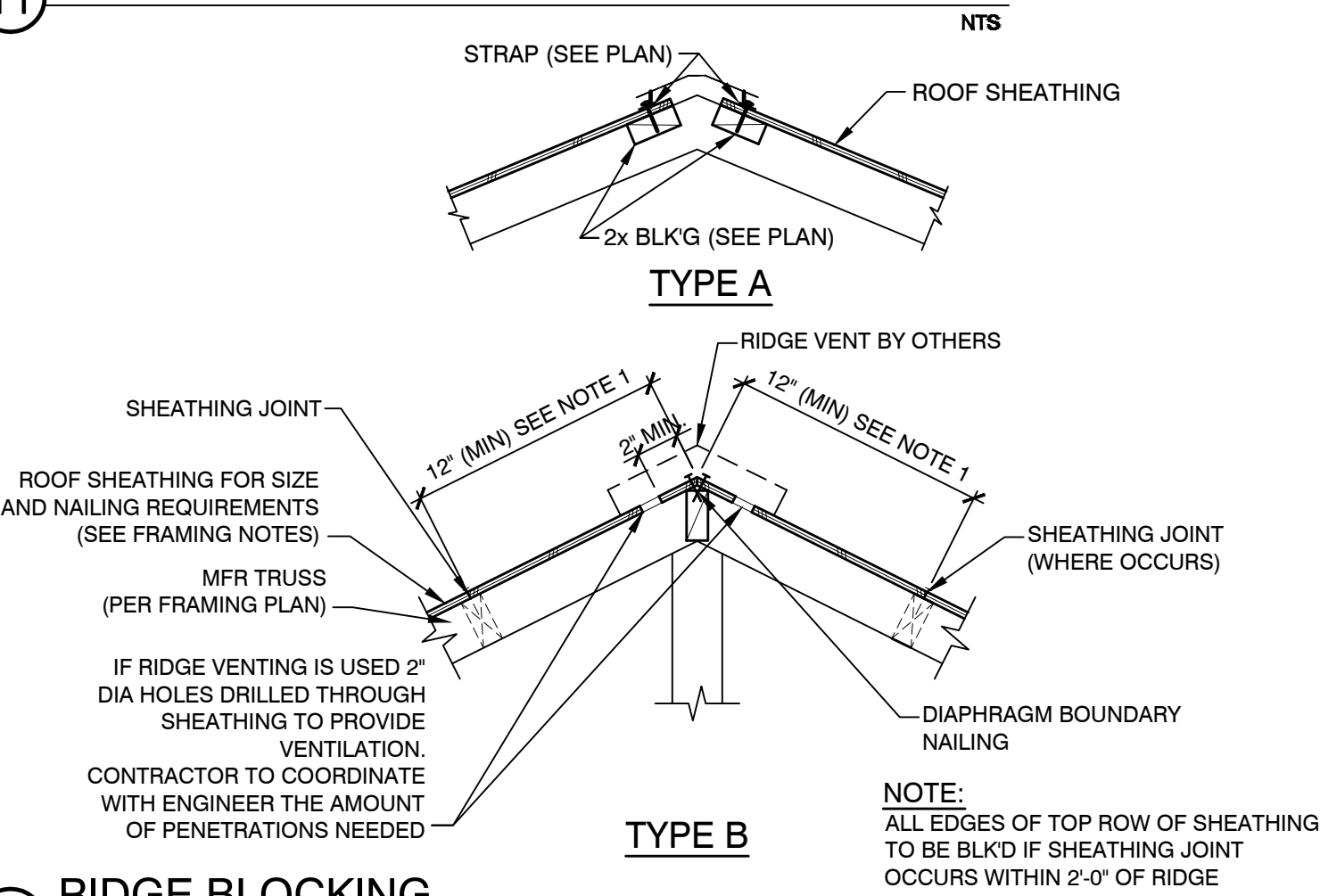
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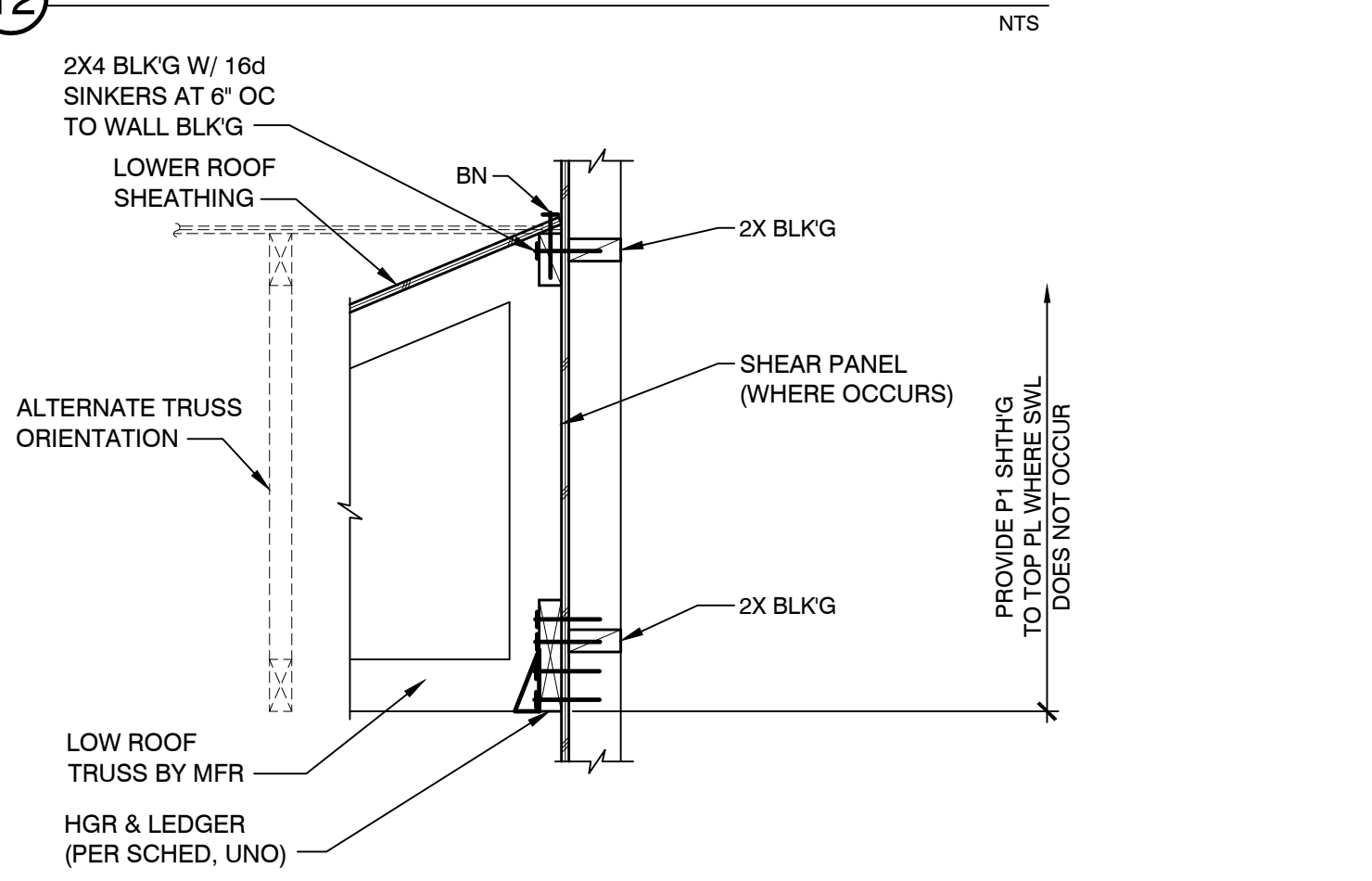
SD-2.1



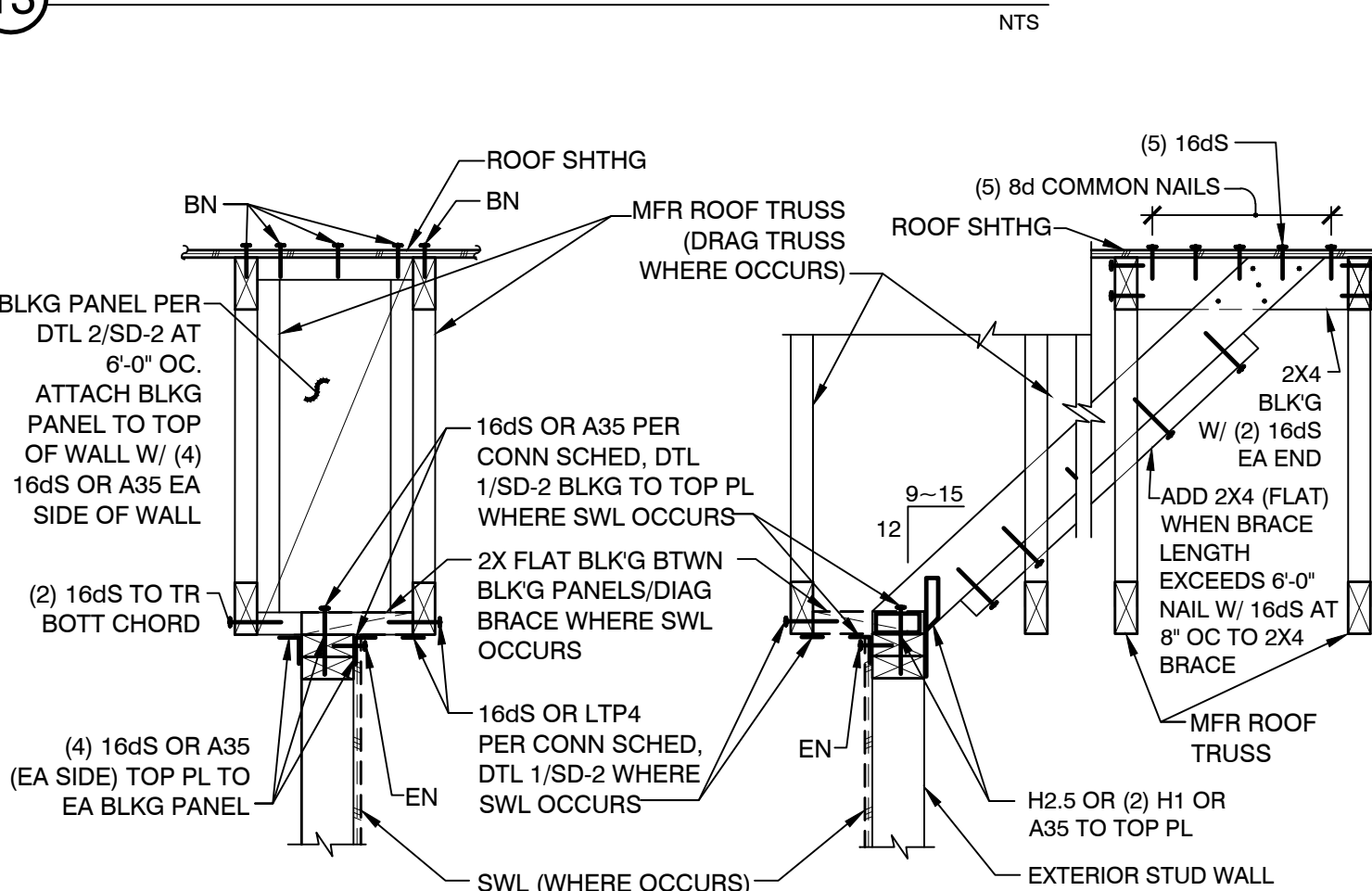
11 BRACING AT NON-BRG WALLS



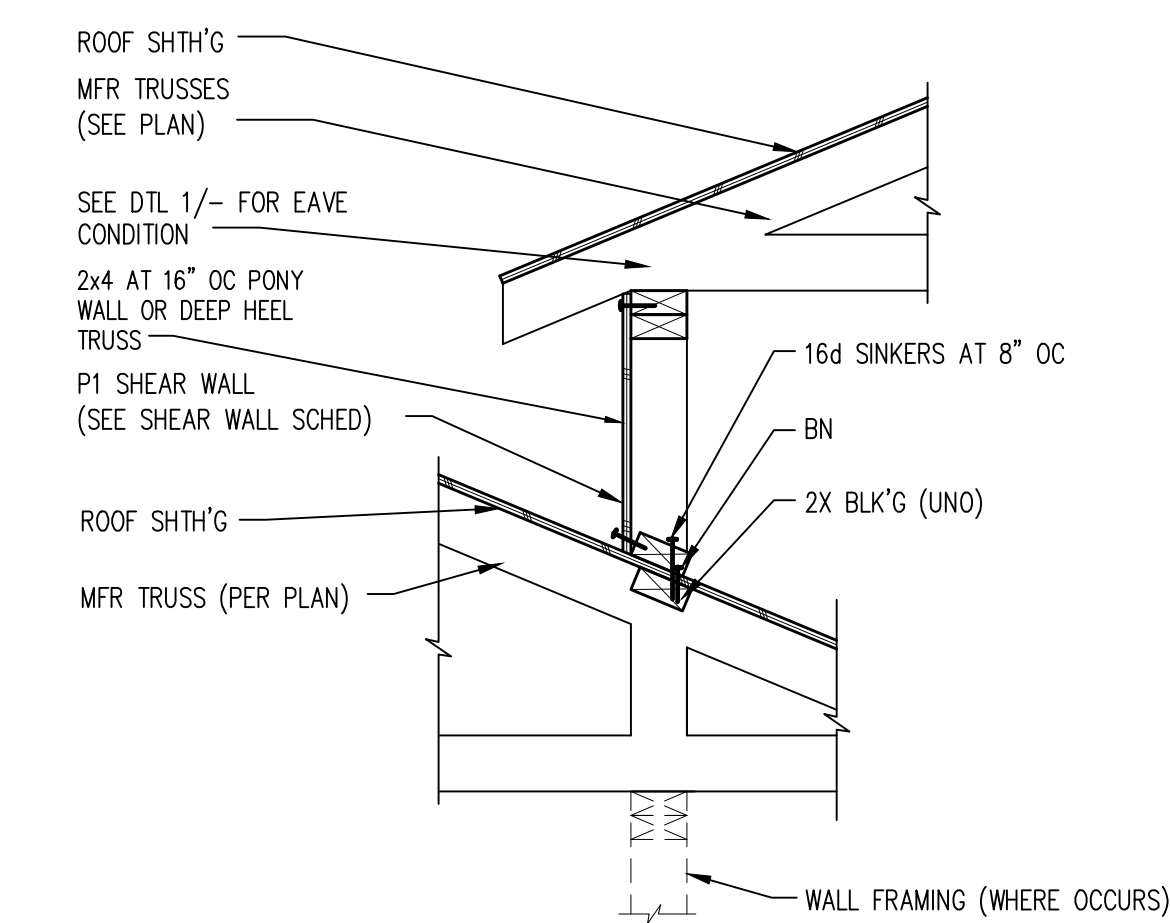
12 RIDGE BLOCKING



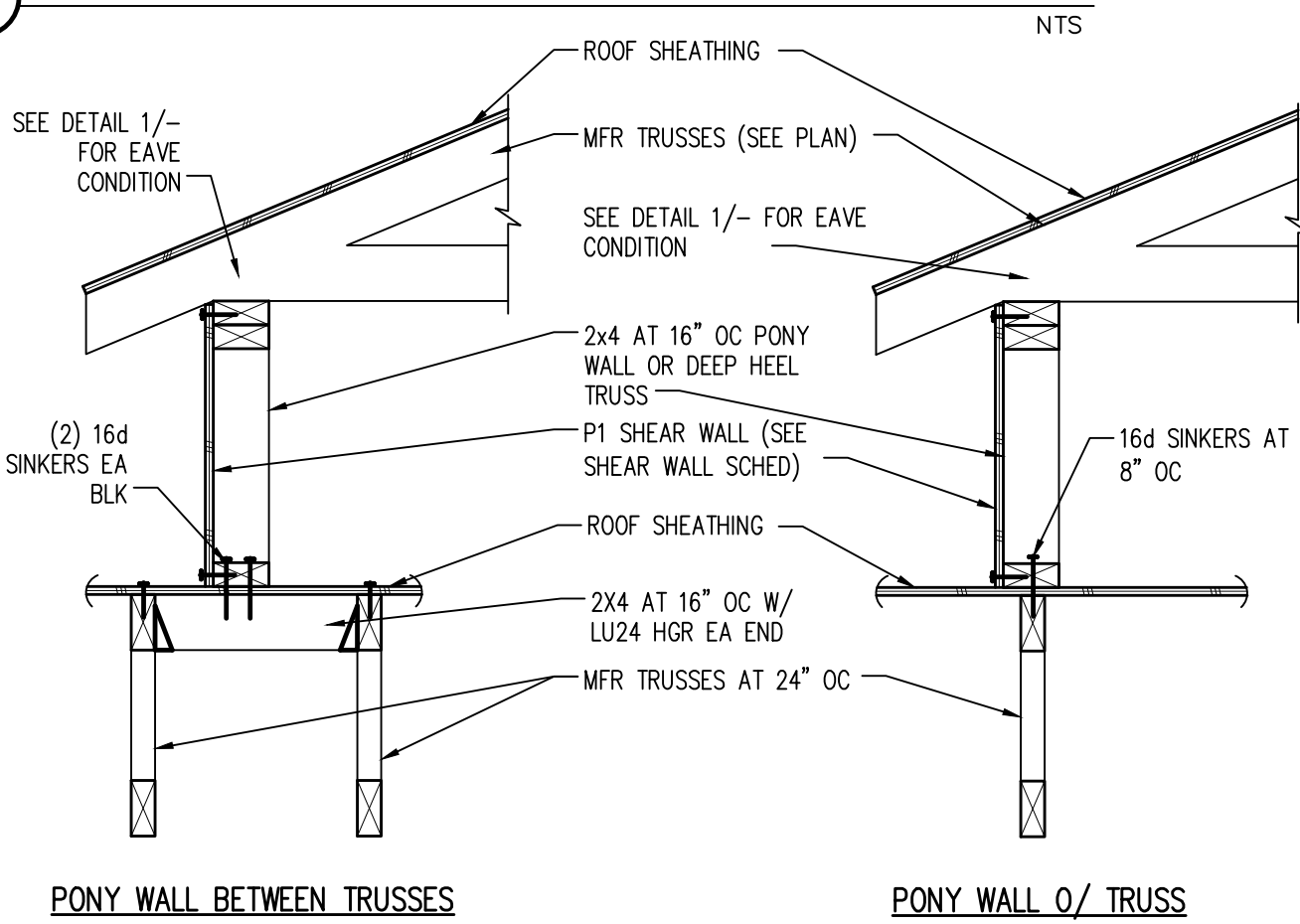
13 LOW ROOF TO WALL



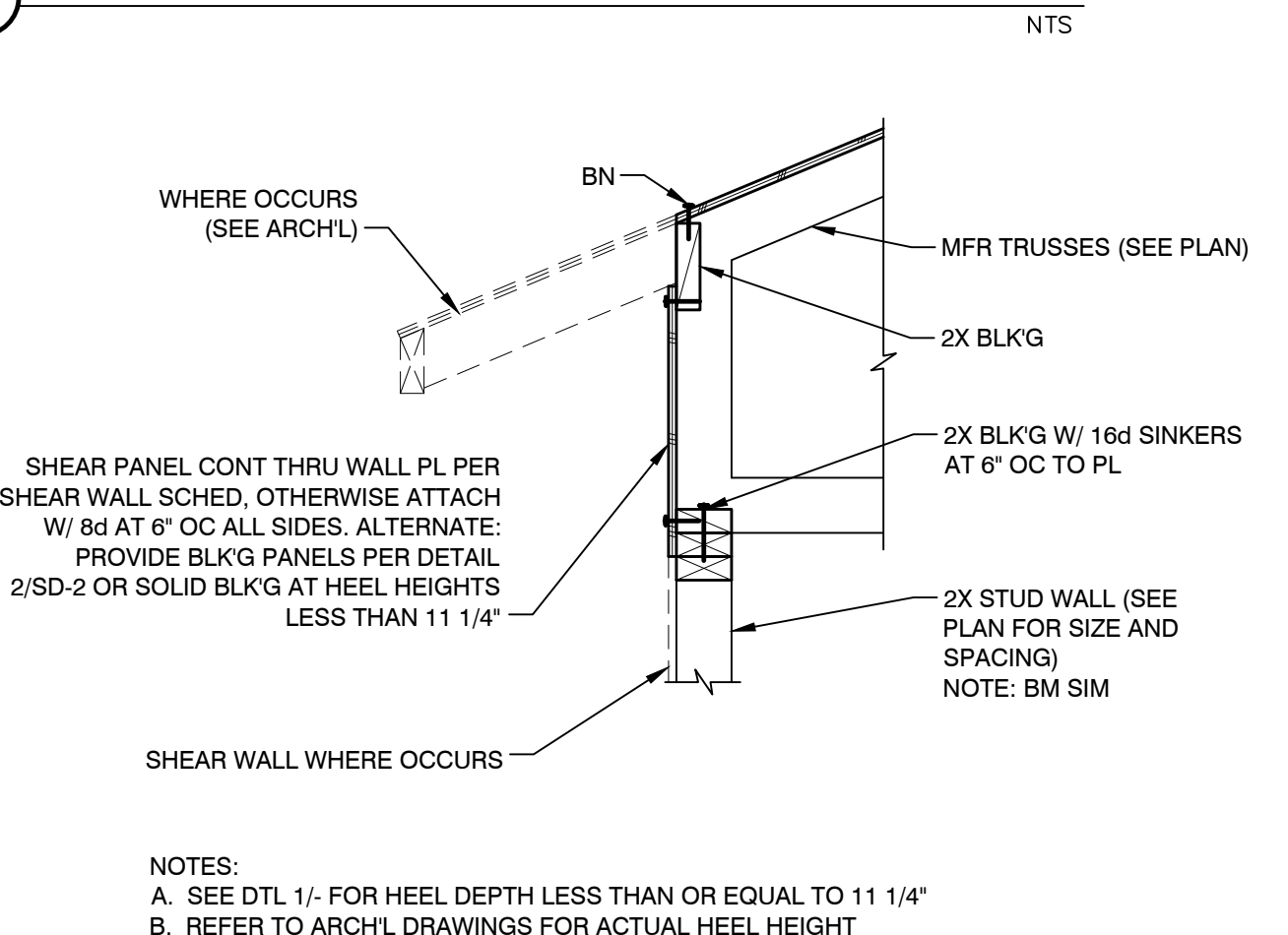
14 EXTERIOR WALL BRACING



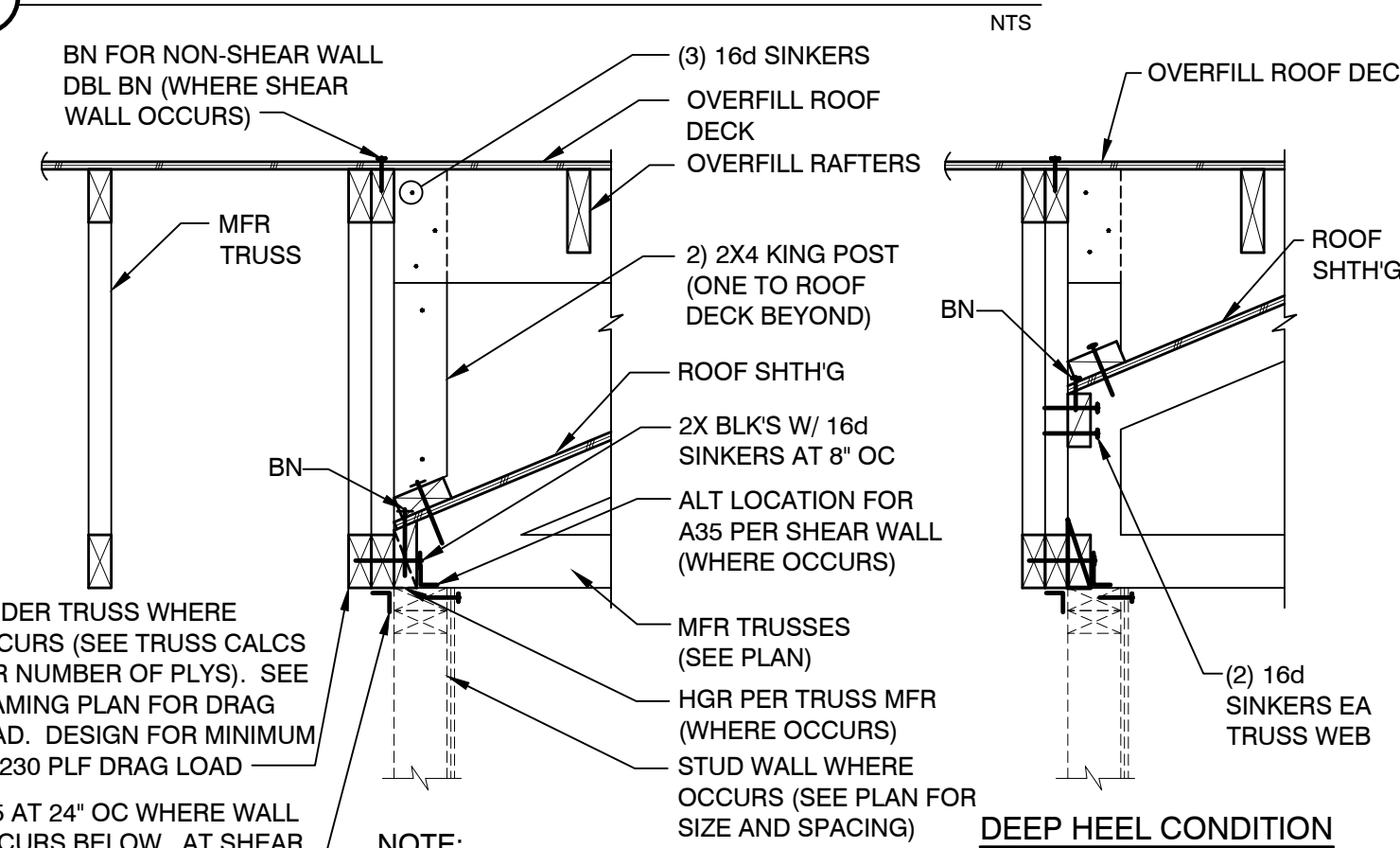
7 PONY WALL



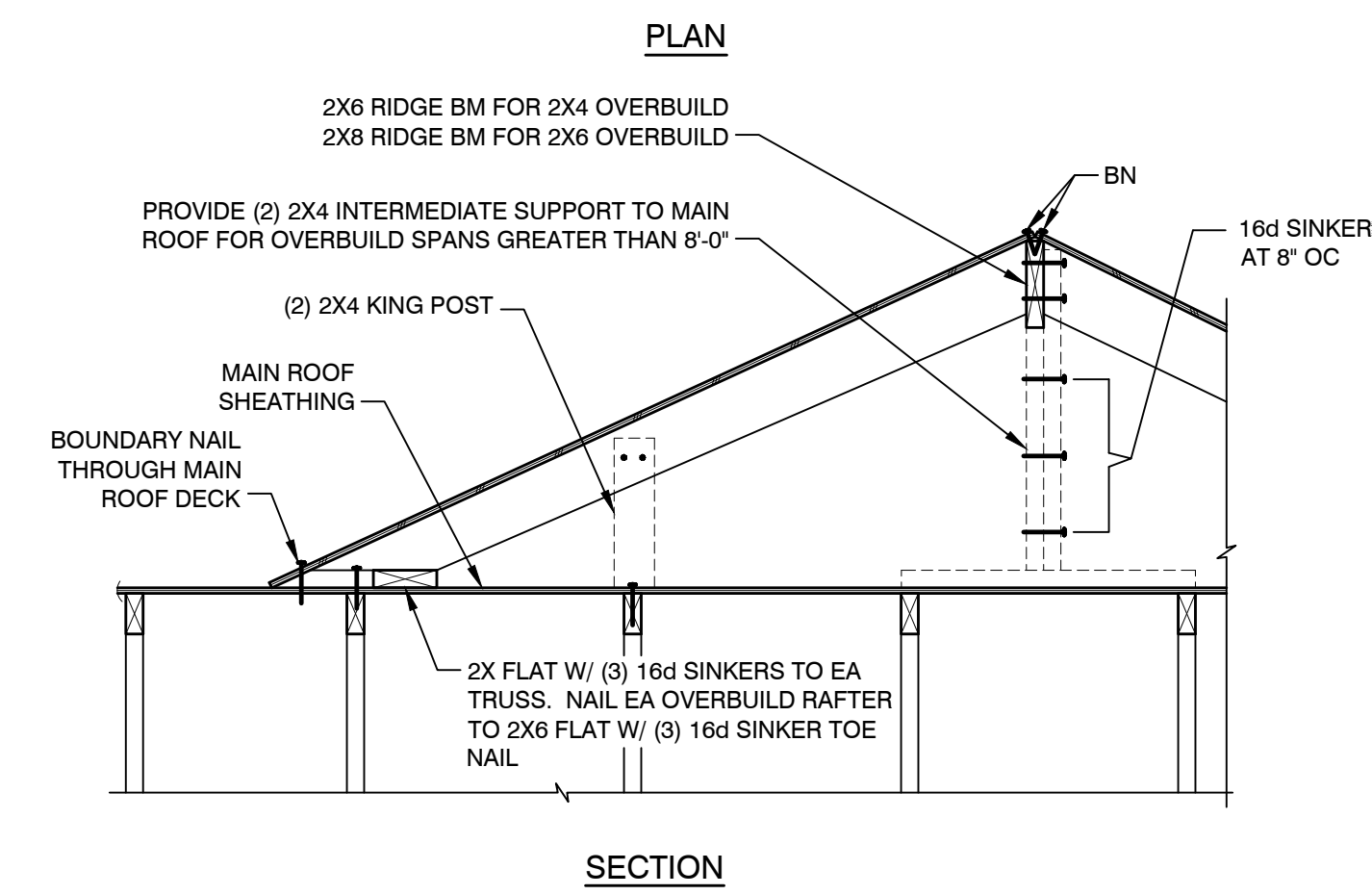
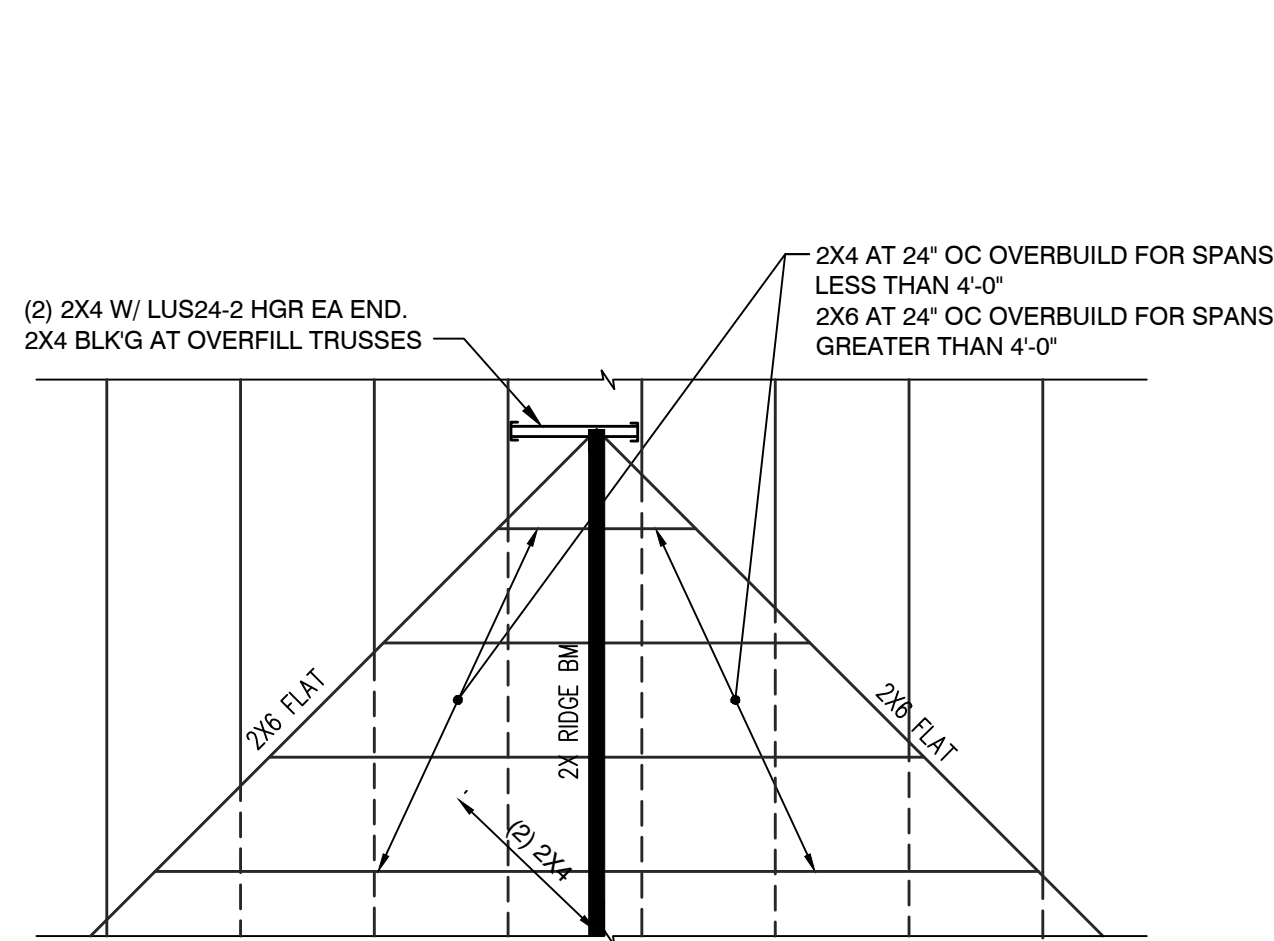
8 PONY WALL



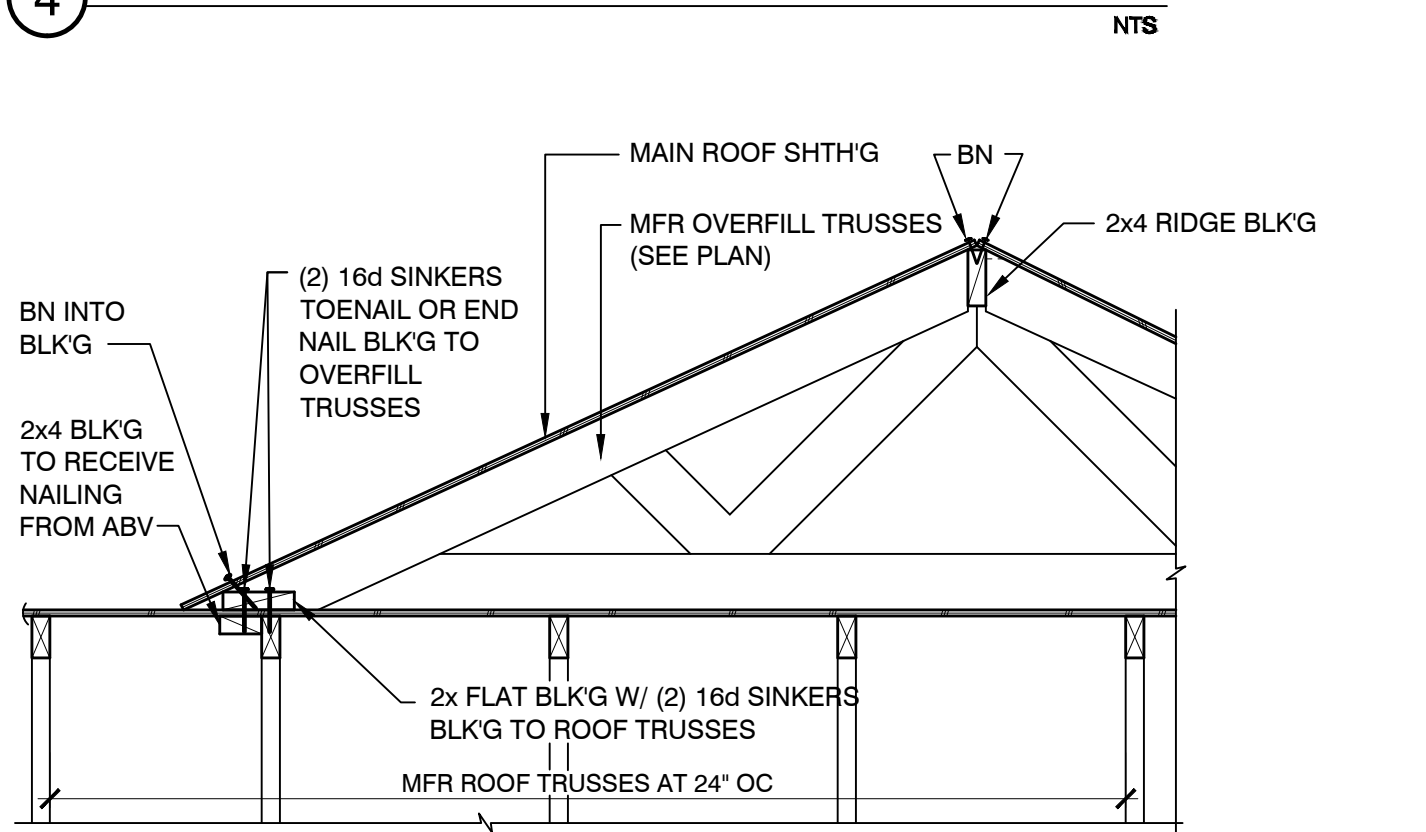
9 TRUSS W/ DEEP HEEL AT BEARING



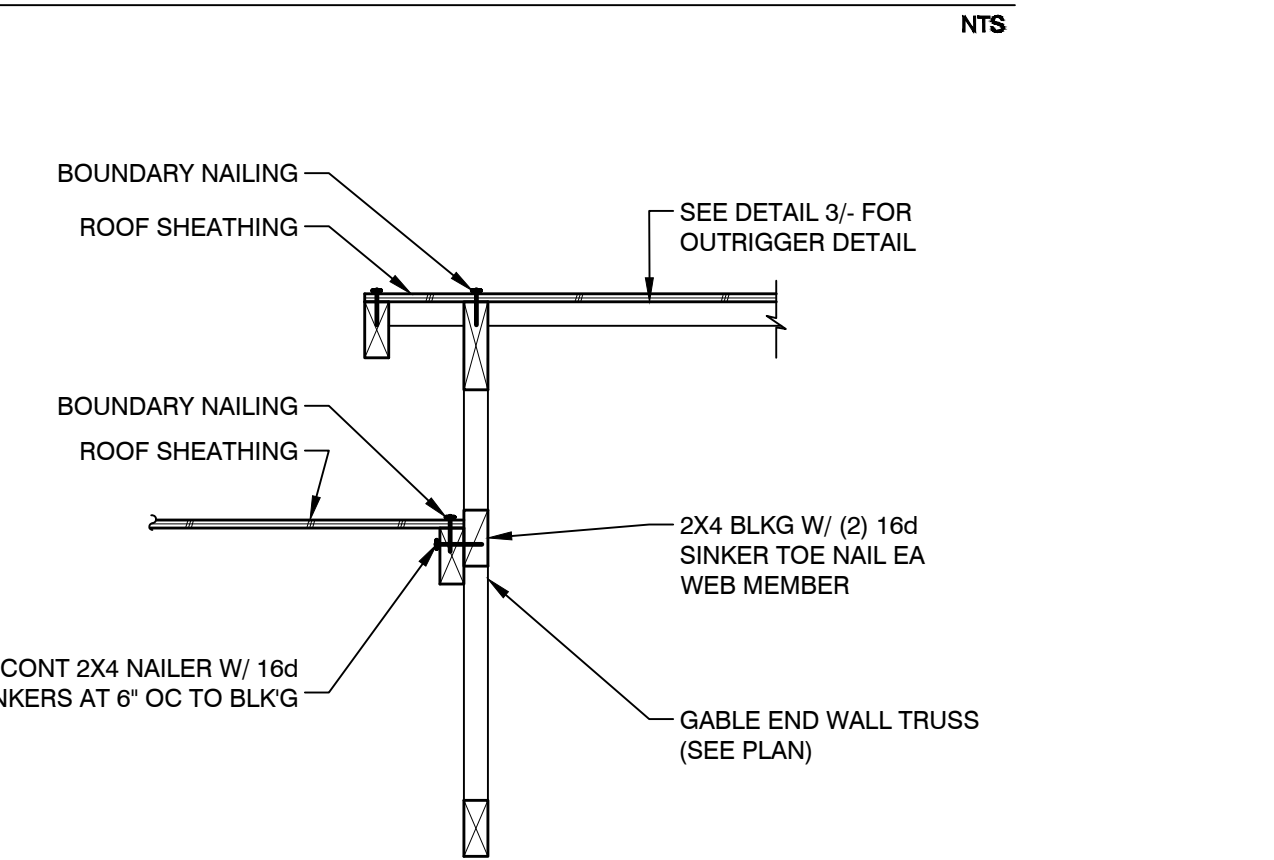
10 TRUSS TO GIRDER



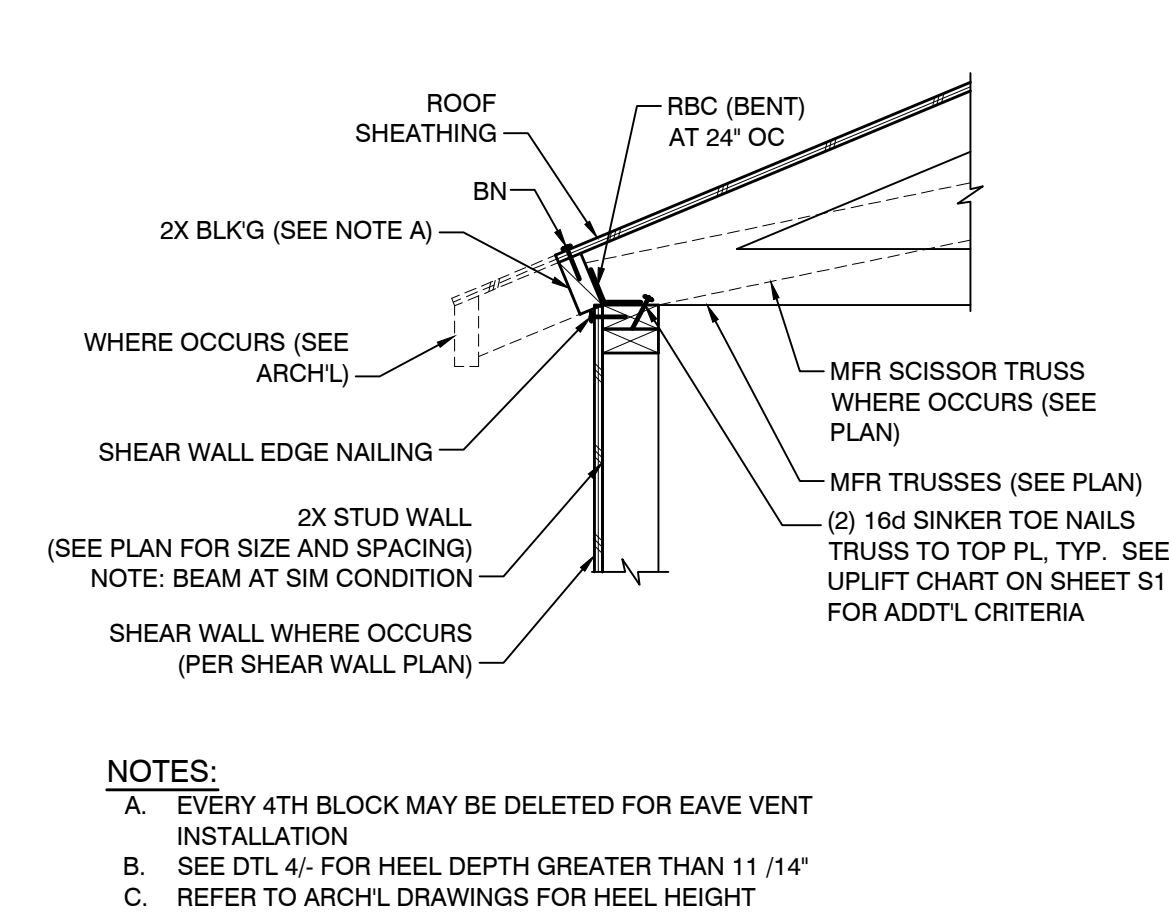
4 OVERFILL FRAMING



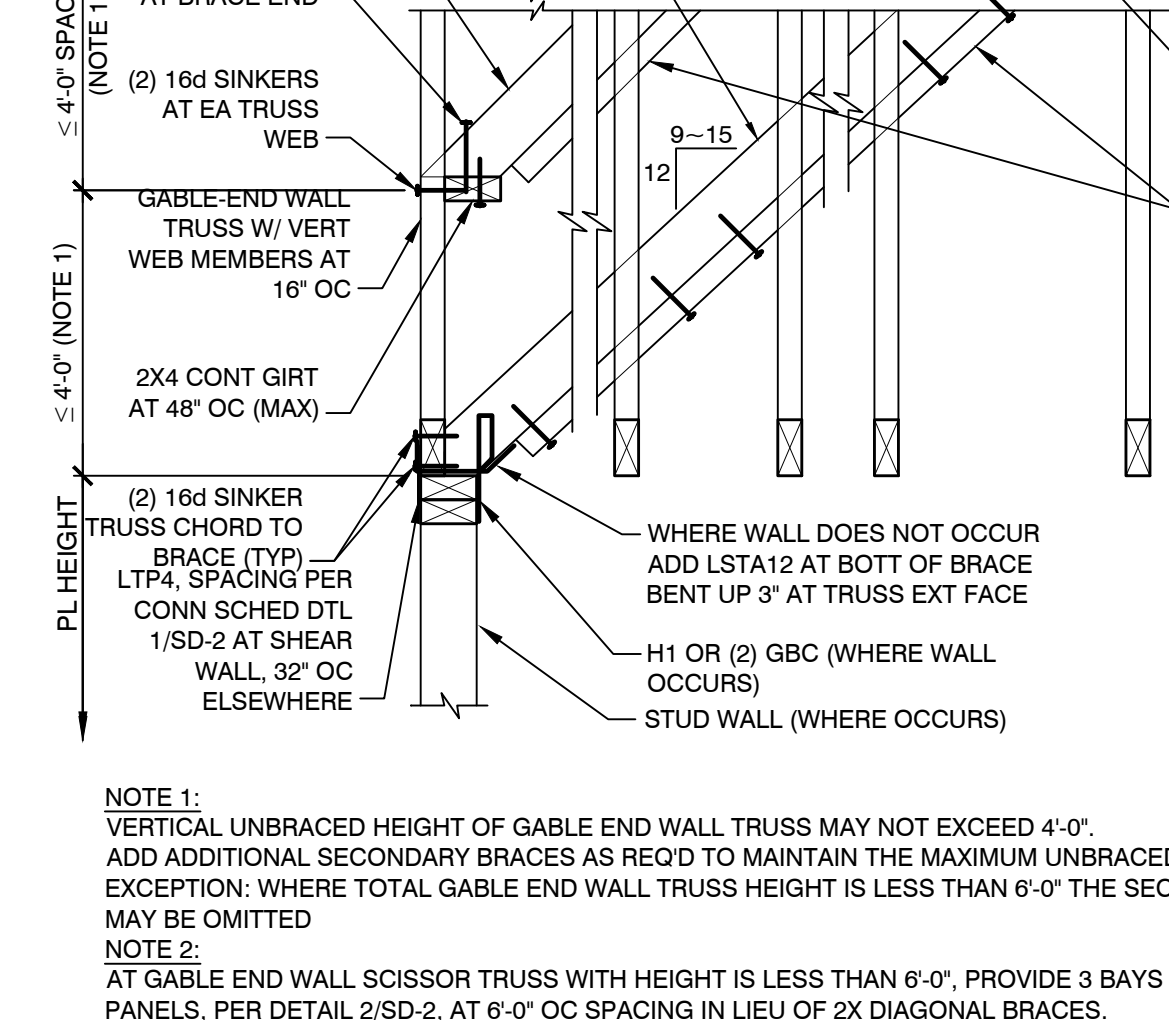
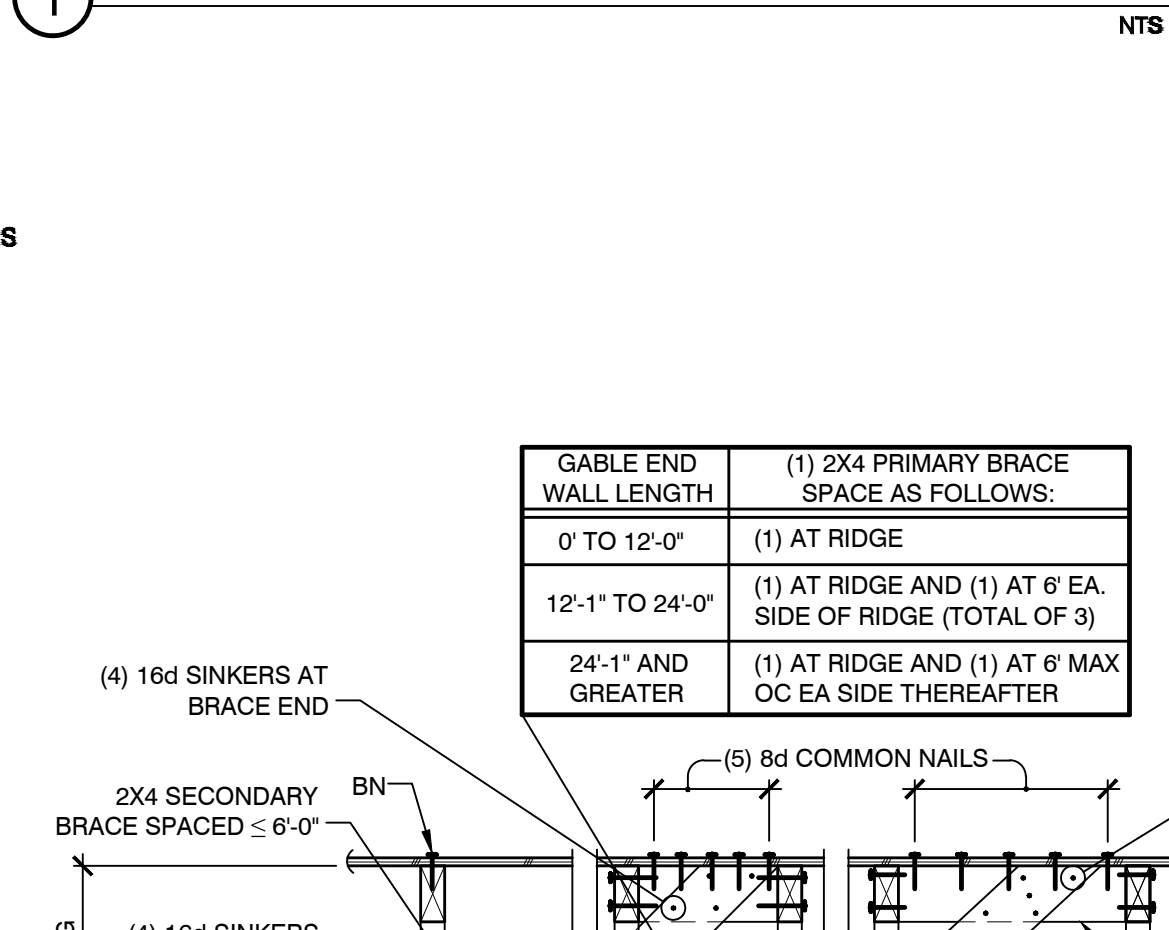
5 OVERFILL FRAMING



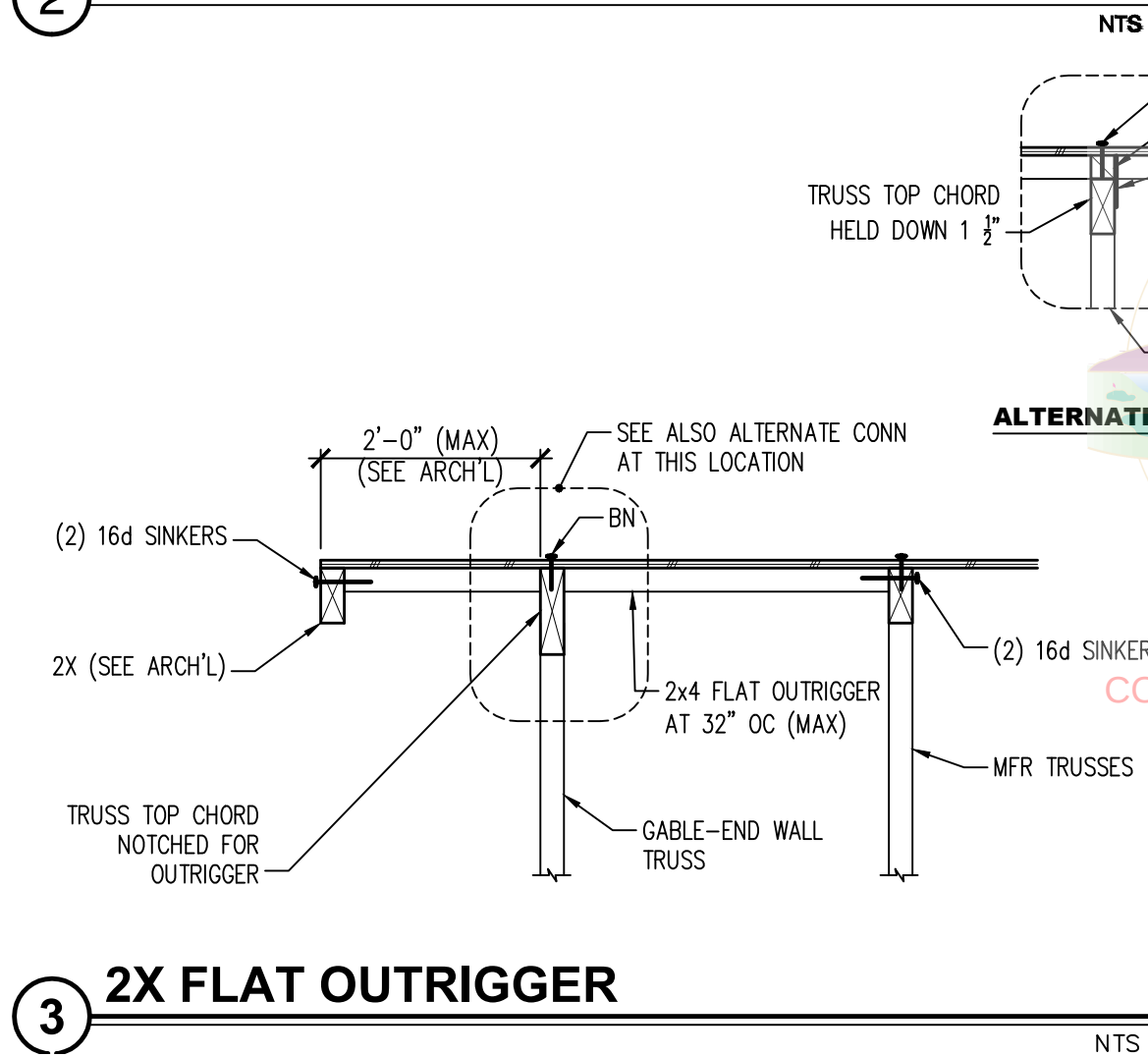
6 STEP IN ROOF



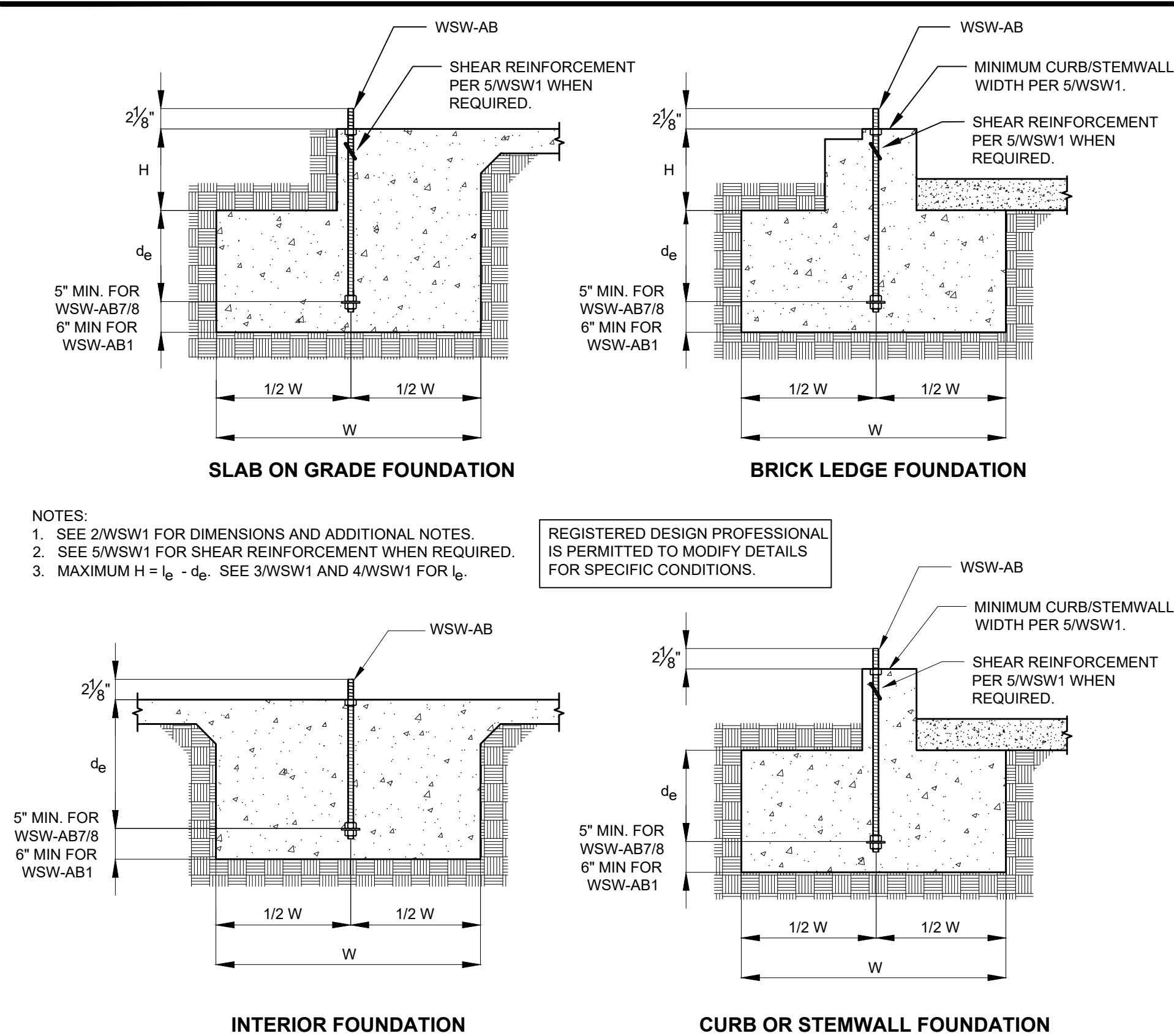
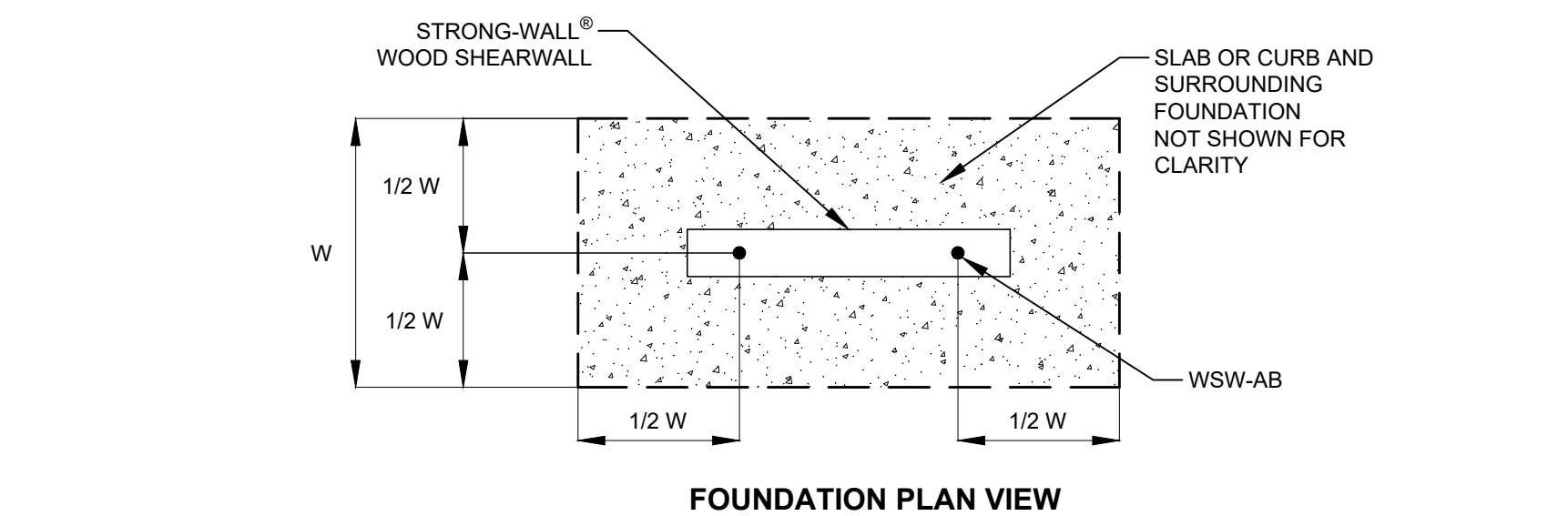
1 EAVE CONDITION



2 GABLE END WALL TRUSS



3 2X FLAT OUTRIGGER

**STRONG-WALL® WSW ANCHORAGE - TYPICAL SECTIONS****1****WSW ANCHOR BOLTS****3****WSW ANCHOR BOLT EXTENSION****4****WSW ANCHOR BOLT TEMPLATES****6**

WSW ANCHORAGE SOLUTIONS FOR 2500 PSI CONCRETE								
DESIGN CRITERIA	CONCRETE CONDITION	ANCHOR STRENGTH	WSW-AB7/8 ANCHOR BOLT			WSW-AB1 ANCHOR BOLT		
			ASD ALLOWABLE TENSION (lb.)	W (in.)	d_e (in.)	ASD ALLOWABLE TENSION (lb.)	W (in.)	d_e (in.)
SEISMIC	CRACKED	STANDARD	11,900	27	9	16,100	33	11
			13,100	29	10	17,100	35	12
		HIGH STRENGTH	24,900	43	15	33,000	51	17
			27,100	46	16	35,300	54	18
	UNCRAKED	STANDARD	12,500	24	8	15,700	28	10
			13,100	25	9	17,100	30	10
		HIGH STRENGTH	25,300	38	13	32,300	44	15
			27,100	40	14	35,300	47	16
WIND	CRACKED	STANDARD	5,100	14	6	6,200	16	6
			8,700	20	7	11,400	24	8
			13,100	27	9	17,100	32	11
			15,900	30	10	21,100	36	12
		HIGH STRENGTH	18,400	33	11	27,300	42	14
			23,100	38	13	31,800	46	16
			27,100	42	14	35,300	50	17
			5,000	12	6	6,400	14	6
	UNCRAKED	STANDARD	9,300	18	6	12,500	22	8
			13,100	23	8	17,100	28	10
			15,200	25	9	21,900	32	11
			19,900	30	10	26,400	36	12
		HIGH STRENGTH	24,000	34	12	31,500	40	14
			27,100	37	13	35,300	43	15

- NOTES:
- ANCHORAGE DESIGNS CONFORM TO ACI 318-11 APPENDIX D AND ACI 318-14 WITH NO SUPPLEMENTARY REINFORCEMENT FOR CRACKED OR UNCRACKED CONCRETE AS NOTED.
 - ANCHOR STRENGTH INDICATES REQUIRED GRADE OF WSW-AB ANCHOR BOLT. STANDARD (ASTM F1554 GRADE 36) OR HIGH STRENGTH (HS) (ASTM A449).
 - SEISMIC INDICATES SEISMIC DESIGN CATEGORY C - F. DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C MAY USE WIND ANCHORAGE SOLUTIONS. SEISMIC ANCHORAGE DESIGNS CONFORM TO ACI 318-11 SECTION D.3.3.4.3 AND ACI 318-14 SECTION 17.2.3.4.3.
 - WIND INCLUDES SEISMIC DESIGN CATEGORY A AND B AND DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C.
 - FOUNDATION DIMENSIONS ARE FOR ANCHORAGE ONLY. FOUNDATION DESIGN (SIZE AND REINFORCEMENT) BY OTHERS. THE REGISTERED DESIGN PROFESSIONAL MAY SPECIFY ALTERNATE EMBEDMENT, FOOTING SIZE OR ANCHOR BOLT.
 - REFER TO 1/WSW1 FOR d_e .

STRONG-WALL® WOOD SHEARWALL TENSION ANCHORAGE SCHEDULE 2,500, 3,000 AND 4,500 PSI**2**

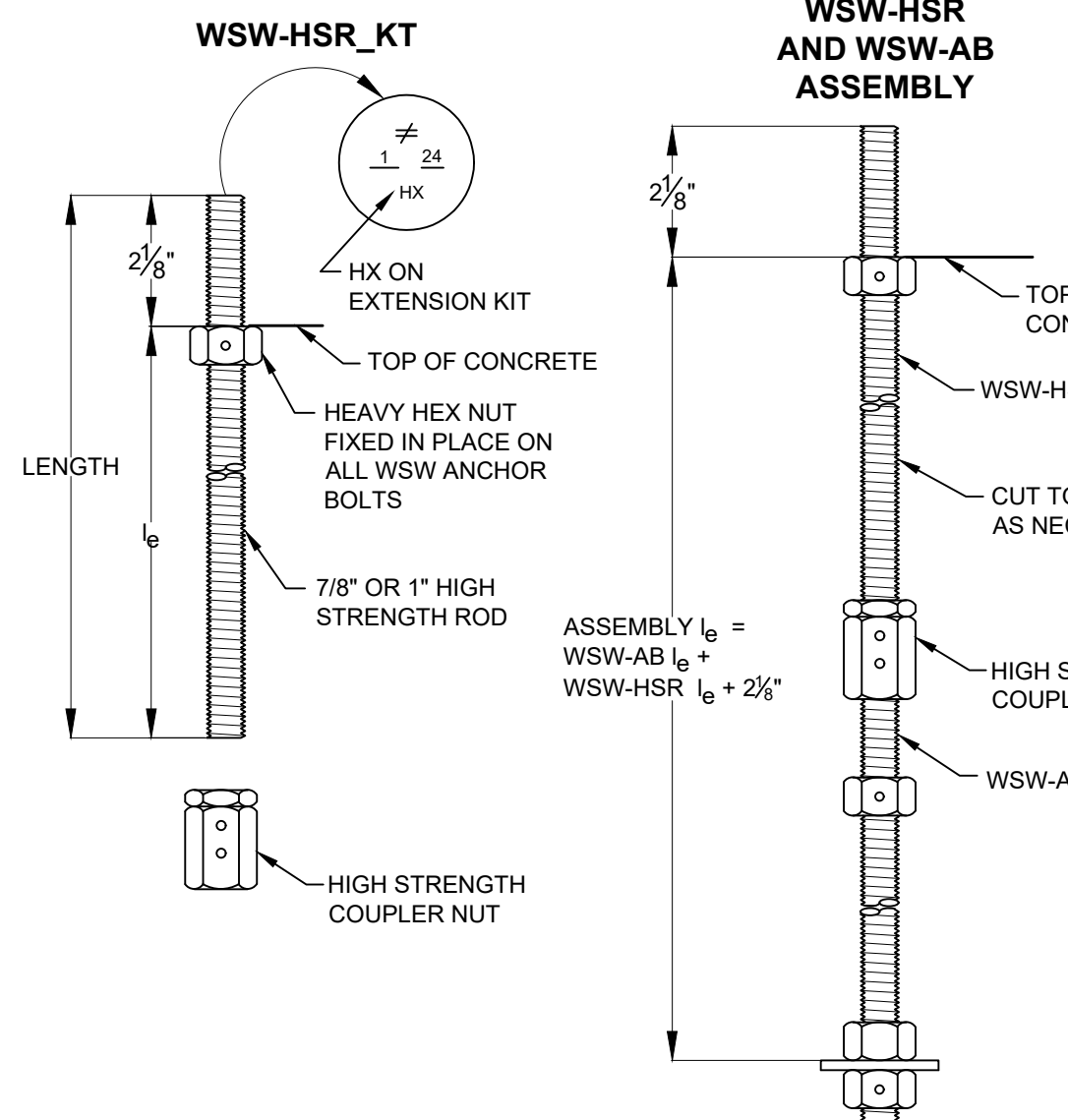
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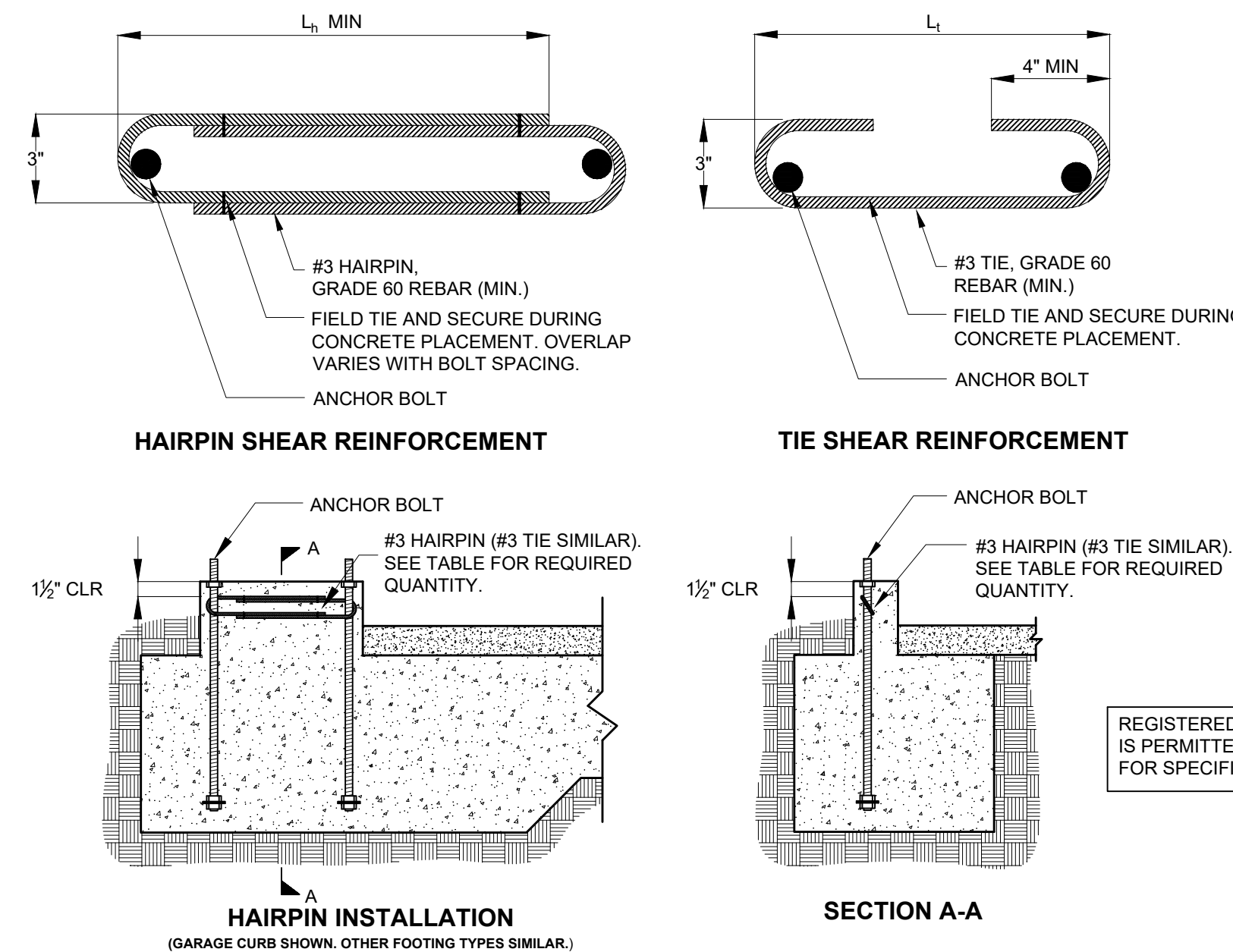
Jul 23, 2021

BY PAUL LUM
INTERWEST CONSULTING GROUP

WSW PANEL MODEL	MODEL NO.	DIAMETER	LENGTH	l_e
WSW12 AND WSW18	WSW-AB7/8x24	7/8"	24"	20"
	WSW-AB7/8x24HS	7/8"	24"	20"
	WSW-AB7/8x30	7/8"	30"	26"
	WSW-AB7/8x30HS	7/8"	30"	26"
	WSW-AB7/8x36HS	7/8"	36"	32"
WSW24	WSW-AB1x24	1"	24"	20"
	WSW-AB1x24HS	1"	24"	20"
	WSW-AB1x30	1"	30"	26"
	WSW-AB1x30HS	1"	30"	26"
	WSW-AB1x36HS	1"	36"	32"

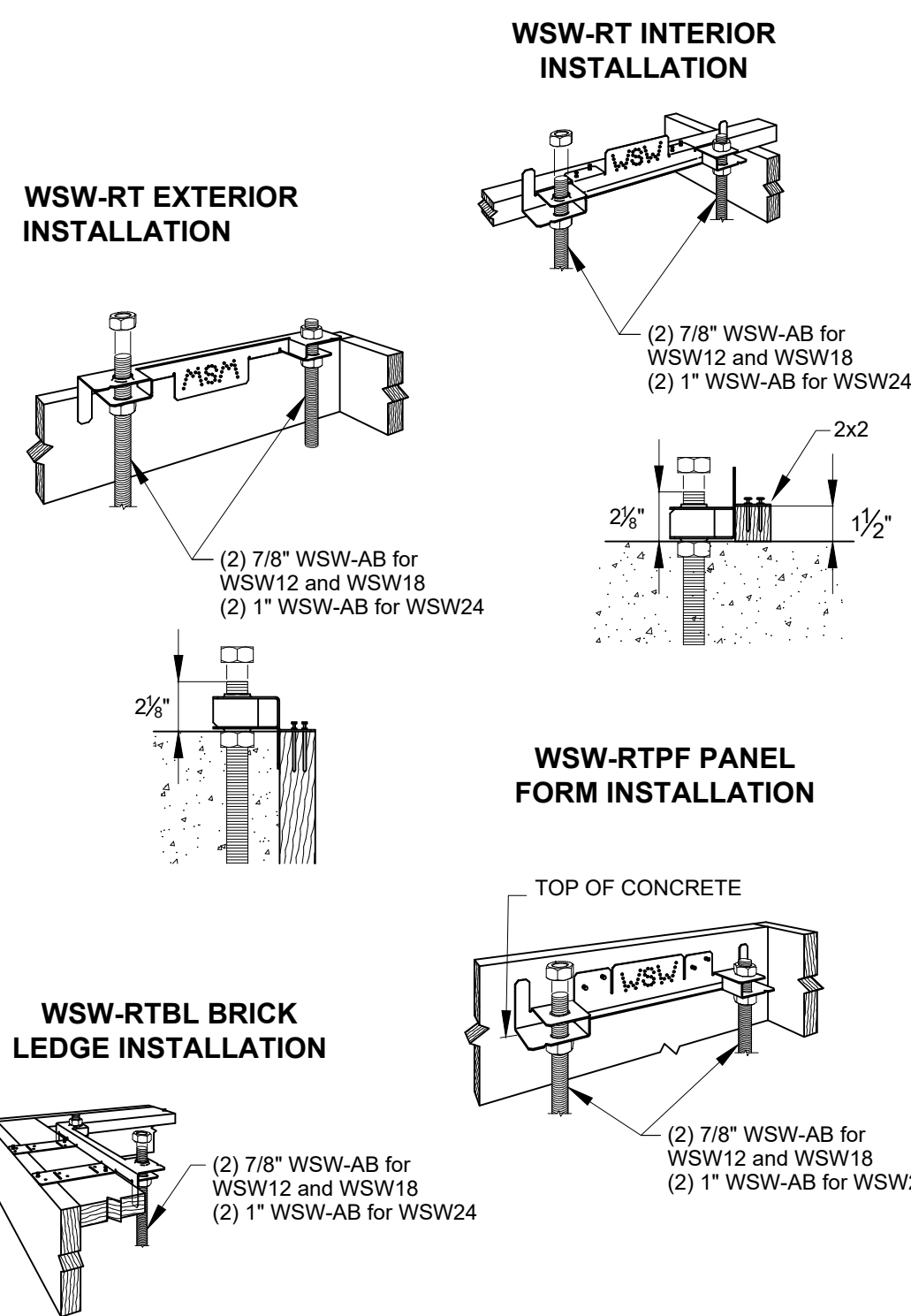


WSW PANEL MODEL	MODEL NO.	DIAMETER	LENGTH	l_e
WSW12 AND WSW18	WSW-HSR7/8x24KT	7/8"	24"	22"
	WSW-HSR7/8x36KT	7/8"	36"	34"
WSW24	WSW-HSR1x24KT	1"	24"	22"
	WSW-HSR1x36KT	1"	36"	34"



STRONG-WALL® WOOD SHEARWALL SHEAR ANCHORAGE						
MODEL	L_t OR L_s (in.)	SEISMIC ³		WIND ⁴		
		SHEAR REINFORCEMENT	MINIMUM CURB/ STEMWALL WIDTH (in.)	SHEAR REINFORCEMENT	MINIMUM CURB/ STEMWALL WIDTH (in.)	ASD ALLOWABLE SHEAR LOAD, V (lb.) ⁵
WSW12	10	(1) #3 HAIRPIN	8"	SEE NOTE 6	6	1,035
WSW18	15	(1) #3 HAIRPIN	8"	(1) #3 HAIRPIN	6	HAIRPIN REINFORCEMENT ACHIEVES MAXIMUM ALLOWABLE SHEAR LOAD OF THE WSW
WSW24	19	(2) #3 HAIRPINS	8"	(1) #3 HAIRPIN	6	

- NOTES:
- SHEAR ANCHORAGE DESIGNS CONFORM TO ACI 318-11 AND ACI 318-14 AND ASSUME MINIMUM 2,500 PSI CONCRETE.
 - SHEAR REINFORCEMENT IS NOT REQUIRED FOR INTERIOR FOUNDATION APPLICATIONS (PANEL INSTALLED AWAY FROM EDGE OF CONCRETE), OR BRACED WALL PANEL APPLICATIONS.
 - SEISMIC INDICATES SEISMIC DESIGN CATEGORY C THROUGH F. DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C MAY USE WIND ANCHORAGE SOLUTIONS.
 - WIND INCLUDES SEISMIC DESIGN CATEGORY A AND B AND DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C.
 - WHERE NOTED, MINIMUM CURB/STEMWALL WIDTH IS 6 INCHES WHEN STANDARD STRENGTH ANCHOR BOLT IS USED.
 - USE (1) #3 TIE FOR WSW12 WHEN PANEL DESIGN SHEAR FORCE EXCEEDS TABULATED ANCHORAGE ALLOWABLE SHEAR LOAD.
 - #4 GRADE 40 SHEAR REINFORCEMENT MAY BE SUBSTITUTED FOR WSW SHEAR ANCHORAGE SOLUTIONS.



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ISSUED FOR: CONSTRUCTION DOCUMENTS

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ASSURED DEVELOPMENT

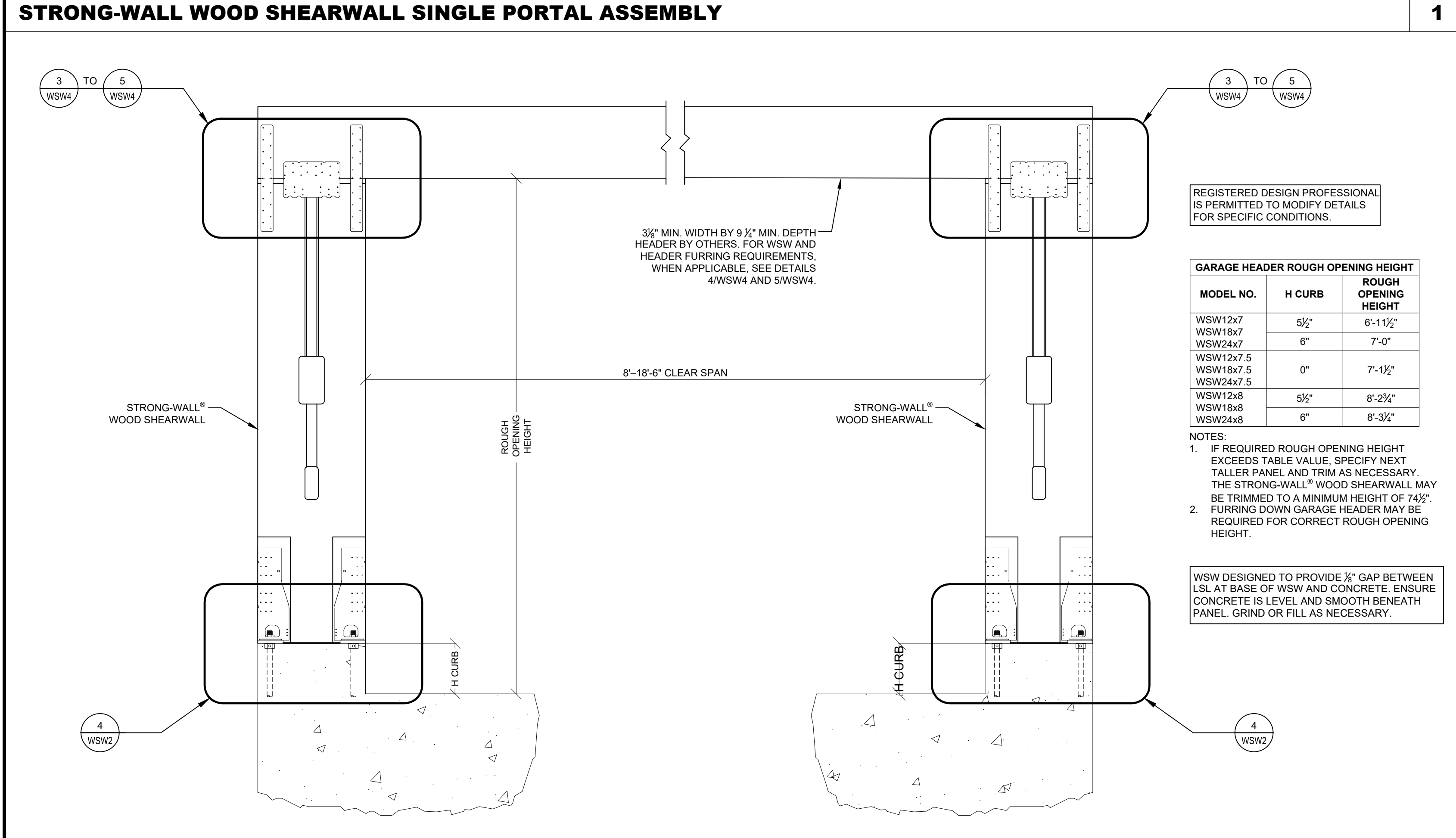
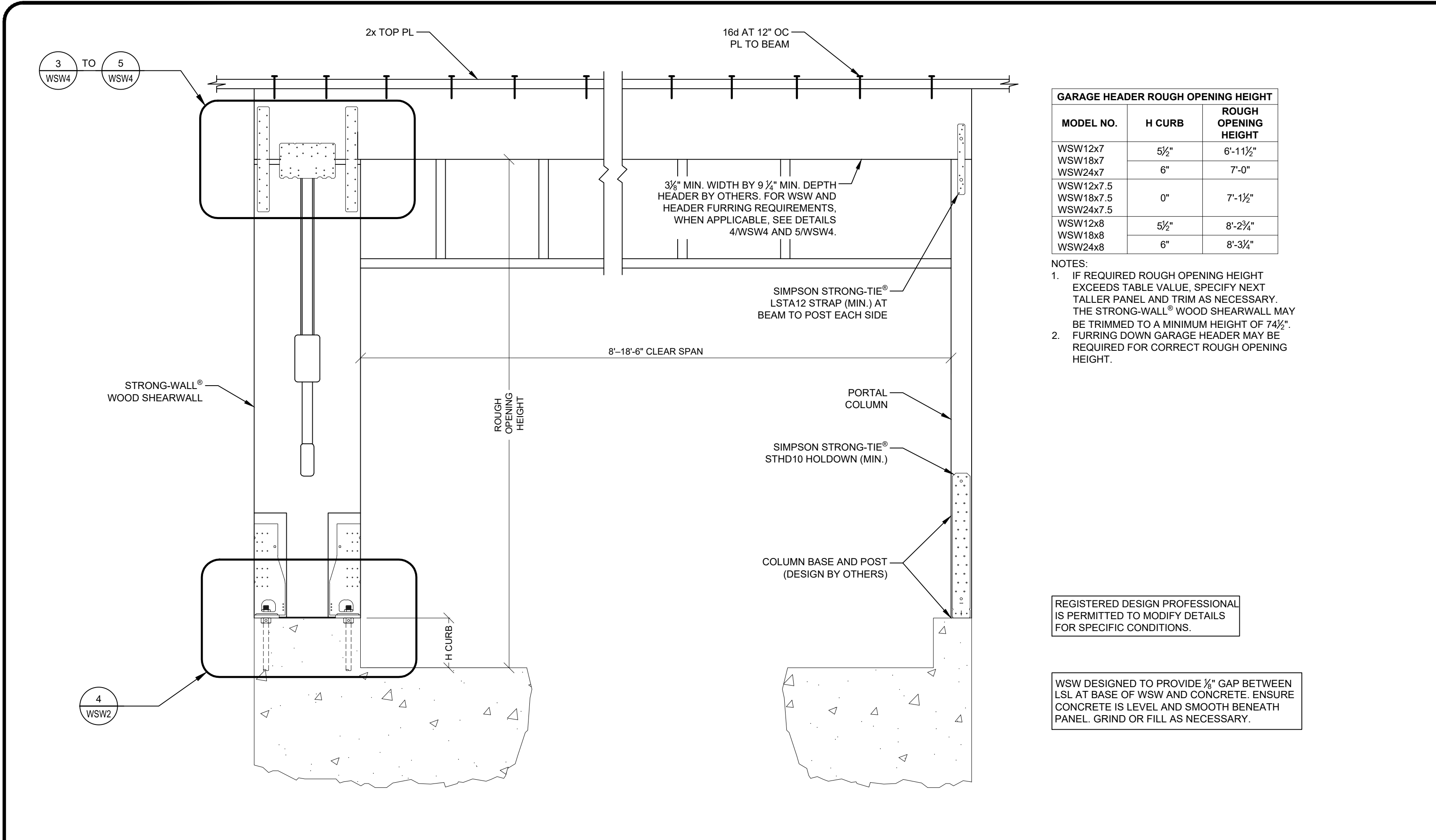
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HENDERSON, NV

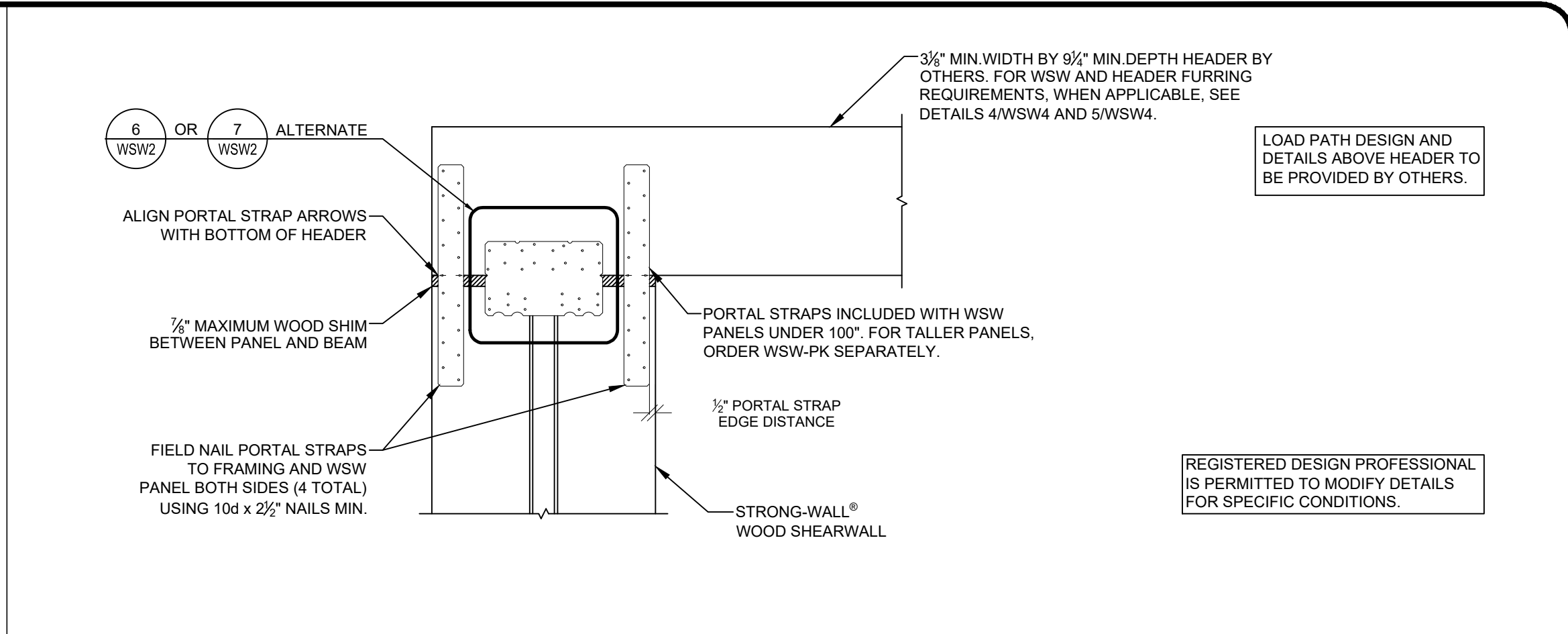
PROFESSIONAL SEAL OF STATE OF NEVADA
KENT A. BARBER
Exp. 12-31-22
CIVIL
No. 017213
03/31/2021

WSW1

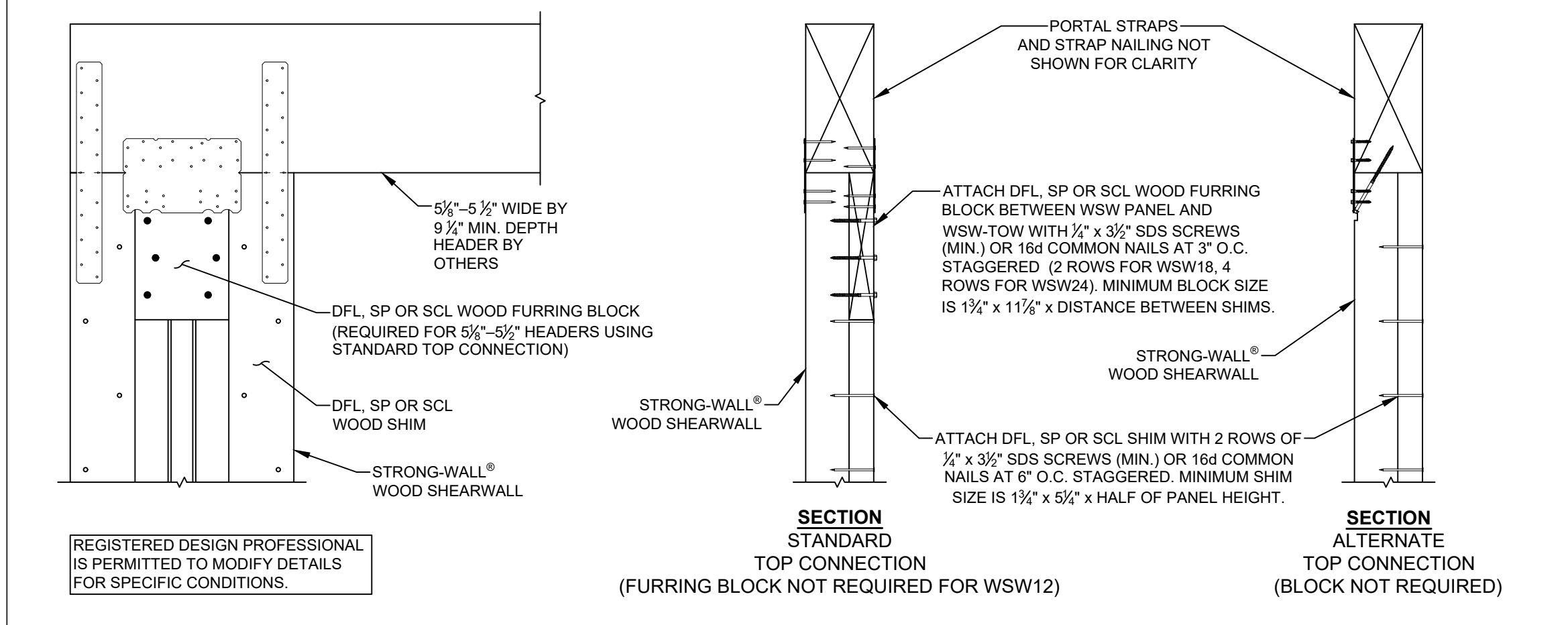
BY PAUL LUM
INTERWEST CONSULTING GROUP



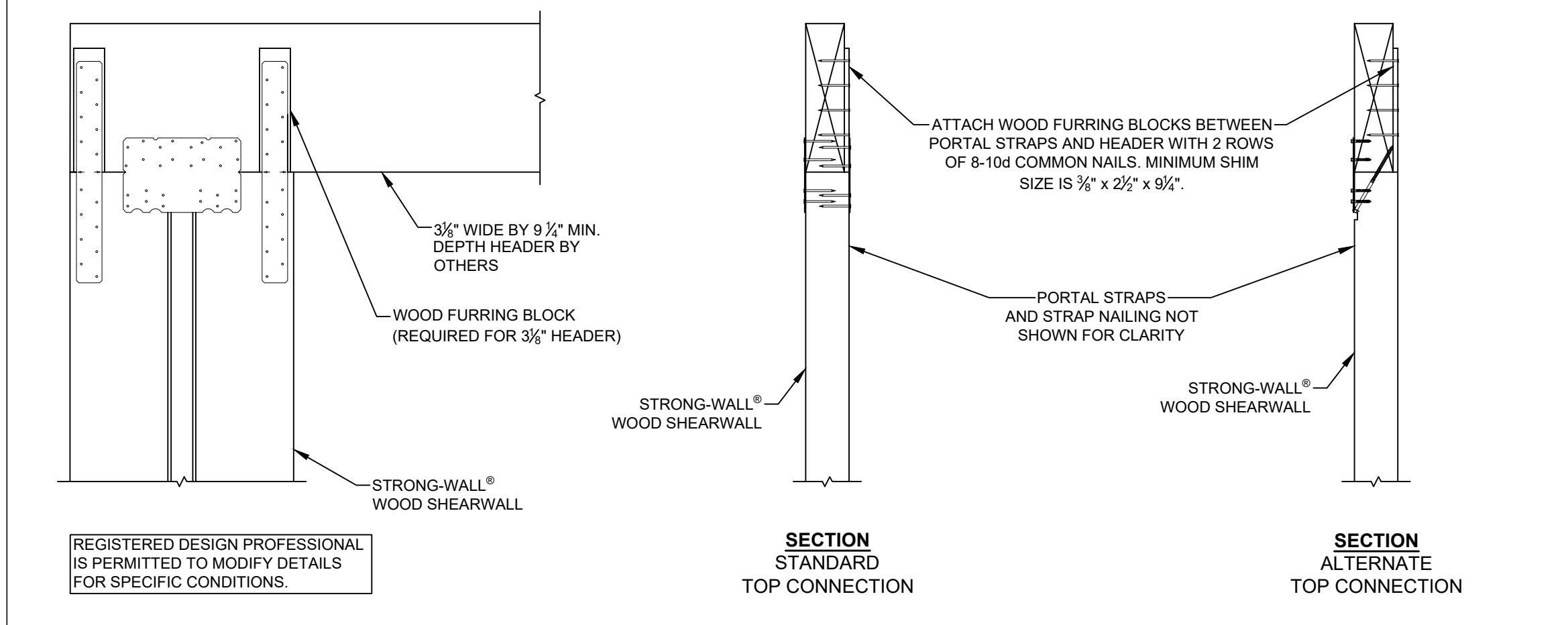
STRONG-WALL WOOD SHEARWALL DOUBLE PORTAL ASSEMBLY



PORTAL TOP CONNECTION



FURRING FOR 5 1/8" TO 5 1/2" HEADER



FURRING FOR 3 1/8" HEADER

1. STRONG-WALL WOOD SHEARWALL IS MANUFACTURED AND TRADEMARKED BY "SIMPSON STRONG-TIE COMPANY INC." HOME OFFICE: 5956 W. LAS POSITAS BLVD., PLEASANTON, CA 94588 TEL: (800) 999-5099, FAX: (925) 847-1597. "SIMPSON STRONG-TIE COMPANY INC." IS AN ISO 9001-2008 REGISTERED COMPANY.

2. USE OF THIS PRODUCT IS SUBJECT TO THE APPROVAL OF THE LOCAL BUILDING DEPARTMENT.

3. THIS PRODUCT IS PART OF THE OVERALL LATERAL FORCE RESISTING SYSTEM OF THE STRUCTURE. DESIGN OF THE BUILDING'S LATERAL FORCE RESISTING SYSTEM, INCLUDING THE LOAD PATH TO TRANSFER LATERAL FORCES FROM THE STRUCTURE TO THE GROUND, IS THE RESPONSIBILITY OF THE DESIGNER.

4. ENGINEER OF RECORD IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS.

5. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONDITIONS, ELEVATIONS, ETC. PRIOR TO INSTALLATION OF ANY COMPONENTS FOR THE STRONG-WALL SB SYSTEM. IF ANY DISCREPANCIES ARE FOUND, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNER FOR CLARIFICATION PRIOR TO CONSTRUCTION.

6. INSTALLATION OF PRODUCT SHALL BE DONE IN CONFORMANCE TO THESE DRAWINGS. THE PERFORMANCE OF MODIFIED PRODUCTS OR ALTERED INSTALLATION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE DESIGNER.

7. SIMPSON STRONG-TIE COMPANY INC. RESERVES THE RIGHT TO CHANGE SPECIFICATIONS, DESIGNS, AND MODELS WITHOUT NOTICE OR LIABILITY FOR SUCH CHANGES.

8. ALL HARDWARE CALLED OUT IS SIMPSON STRONG-TIE.

9. SEE ICC-ES ESR-2652 OR CITY OF LOS ANGELES RR25730 AS APPLICABLE FOR ADDITIONAL INFORMATION.

THE CITY OF
HENDERSON
A Place To Call Home
COH APPROVED - 08/17/2021

NOTES